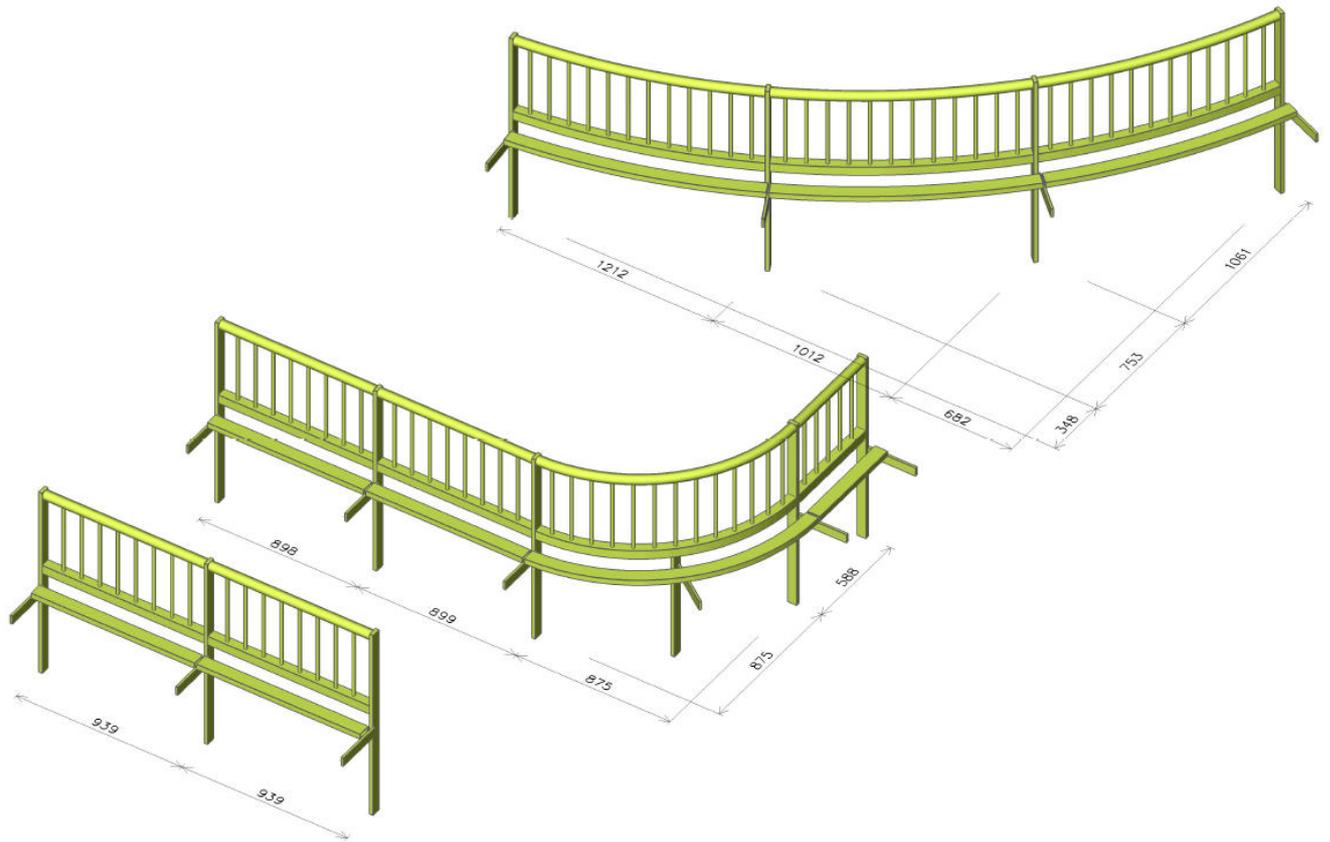


PROJECT <b>SAMPLE PROJECT IN LONDON</b>	ENGINEER
DOCUMENT NO. <b>STR-CALC-615</b>	REVISION <b>0</b>
TITLE <b>METAL BALUSTRADES</b>	Pages <b>57</b>

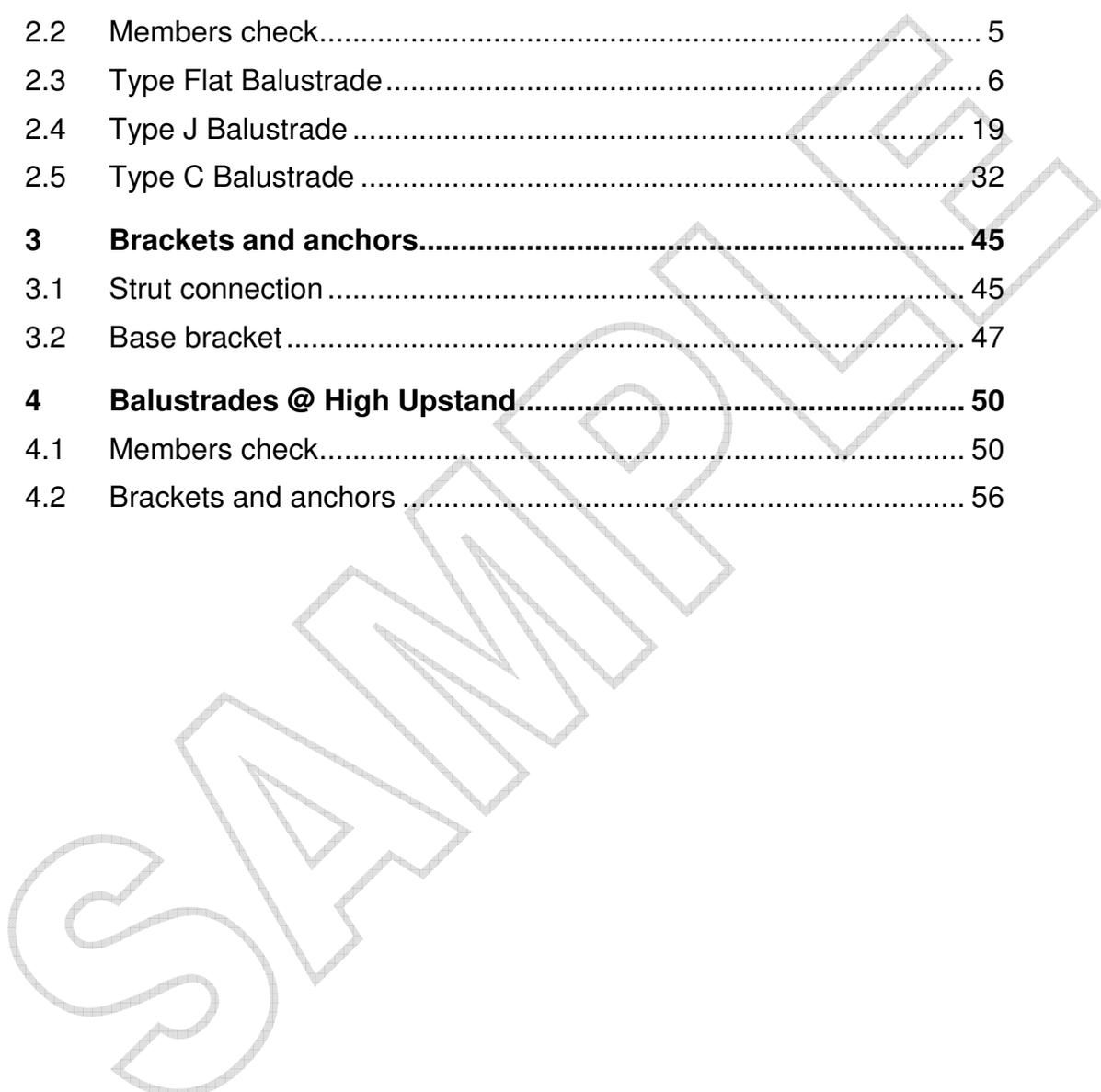


SALE

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# 1 Basic Data

## 1.1 References

### 1.1.1 Norms and Standards

- [1] BS EN 1990:2002, Eurocode – Basis of structural design.
- [2] BS EN 1993-1, Eurocode 3 – Design of steel structures – Parts 1, 4 & 8.
- [3] BS EN 1999-1, Eurocode 9 – Design of aluminium structures – Parts 1 & 4
- [4] CWCT:2005, Standard for systemised building envelopes, Parts 2 & 3.
- [5] CWCT TU14, Technical update on Load combinations.

### 1.1.2 Document Reference

- [6] Lindner. *D262-LFL-SW-TYP-CL-F-1401: Loads and load combinations.*
- [7] Wintech. *Performance Specification for the Installation of Curtain wall.* Rev. M, 24 October 2014.

### 1.1.3 Software

- [8] Nemetschek. *SCIA Engineer v.14.0.* Structural Analysis & Design Software for Construction and Engineering.

## 1.2 Loads

The following loads are in accordance with load report [6].

### 1.2.1 Dead Load (D)

Selfweight of the framing profiles are generated by the software Scia [8].

### 1.2.2 Imposed/live load (L)

The most onerous of the following when combined with other loads in accordance with load report [6].

#### i Vertical load to internal ledges and horizontal members/surfaces

$$\text{Point load, } Q_{Iv,k} = 1.0 \text{ kN}$$

$$\text{Uniform pressure, } w_{Iv,k} = 0.6 \text{ kN/m}^2$$

#### ii Barrier horizontal loads

$$\text{Line load, } q_{Ih,k} = 0.74 \text{ kN/m} \text{ - applied at a height of 1.1m above FFL}$$

$$\text{Point load, } Q_{Ih,k} = 0.5 \text{ kN} \text{ - applied within a height of 1.1m above FFL}$$

$$\text{Infill load, } w_{Ih,k} = 1.0 \text{ kN/m}^2 \text{ - applied within a height of 1.1m above FFL}$$

### 1.2.3 Wind load (W)

$$\text{Net pressure, } w = \pm/-2.04 \text{ kN/m}^2$$

### 1.2.4 Thermal load

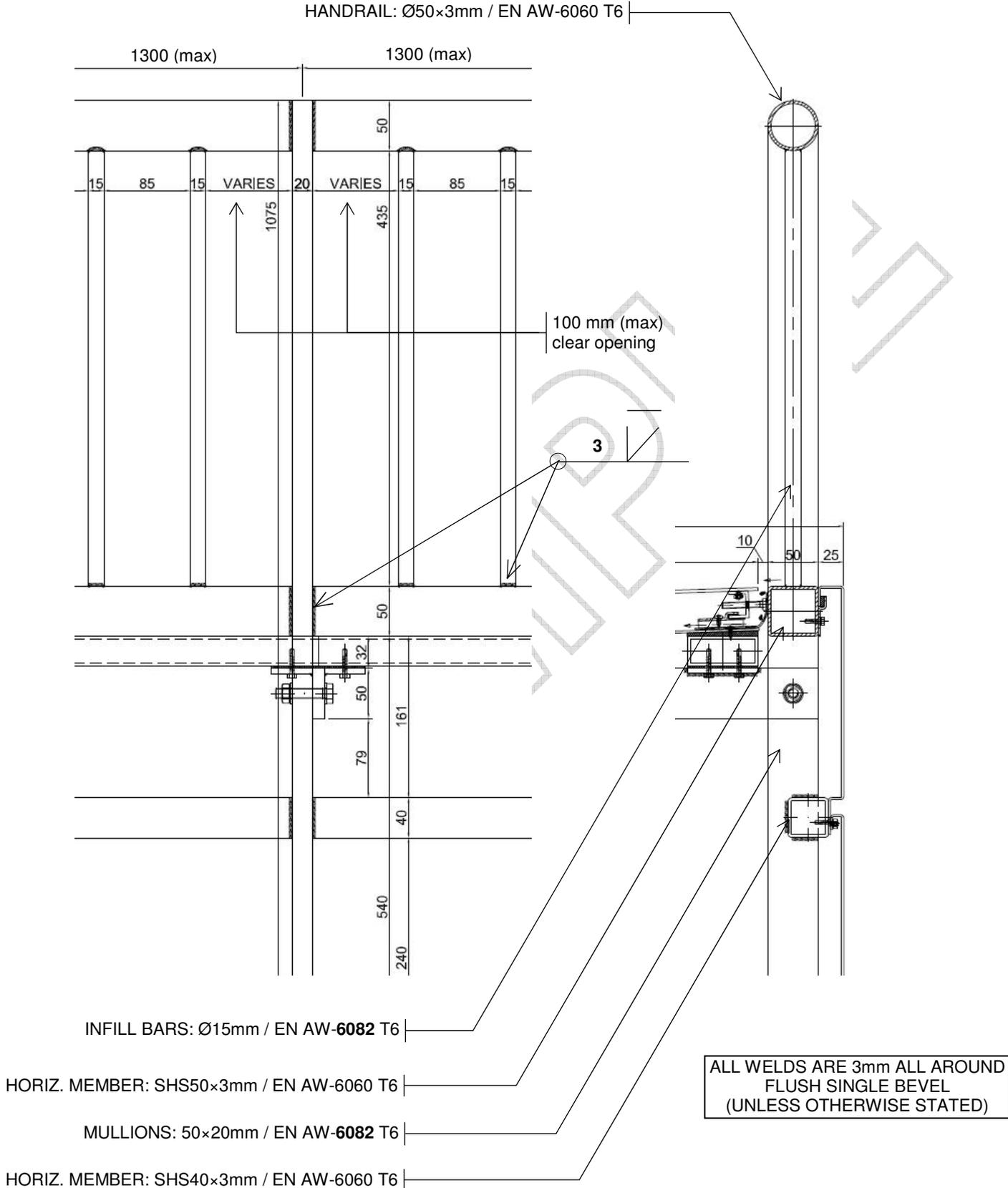
$$\text{Summer, } T_1 = +65 \text{ }^\circ\text{K}$$

$$\text{Winter, } T_2 = -25 \text{ }^\circ\text{K}$$

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## 2 Typical balustrade

### 2.1 Typical Detail



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## 2.2 Members check

### 2.2.1 Handrail

$$\begin{aligned} \text{Max. span, } L &= 1.3 \text{ m} \\ M_{Ed} &= 1.5 \cdot 0.74 \cdot 1.3^2 / 8 = 0.23 \text{ kN}\cdot\text{m} \end{aligned}$$

#### Ø50×3mm / EN AW-6060 T6

$$\begin{aligned} W_{el} &= 4.8 \text{ cm}^3 \\ I &= 11.6 \text{ cm}^4 \\ M_{el,Rd} &= 4.8 \cdot 140 / 1.1 = 1.32 \text{ kN}\cdot\text{m} \quad \underline{0.17 < 1.0} \\ \delta &= 5 \cdot 0.74 \cdot 1300^4 / (384 \cdot 210000 \cdot 116000) \\ &= 1.13 \text{ mm} \end{aligned}$$

### 2.2.2 Mullion

$$\begin{aligned} \text{Cantilever, } L &= 0.65 \text{ m} \\ H_k &= \max\{0.74 \cdot 1.3; 0.5\} = 0.96 \text{ kN} \\ M_{Ed} &= 1.5 \cdot 0.96 \cdot 0.65 = 0.94 \text{ kN}\cdot\text{m} \end{aligned}$$

#### Flat 50×20mm / EN AW-6082 T6

$$\begin{aligned} M_{pl,Rd} &= 1.2 \cdot 20 \cdot 50^2 / 6 \cdot 250 / 1.1 = 2.27 \text{ kN}\cdot\text{m} \quad \underline{0.41 < 1.0} \\ \delta &= 960 \cdot 600^3 / (3 \cdot 70000 \cdot 12 \cdot 50^3 / 20) \\ &= 2.63 \text{ mm} \end{aligned}$$

### 2.2.3 Check overall deflection

$$\begin{aligned} \text{Overall, } \delta &\leq \min\{600/65; 15\} = 9.23 \text{ mm} \\ \text{Overall, } \delta &= 1.13 + 2.63 = 3.76 \text{ mm} \quad \underline{0.41 < 1.0} \end{aligned}$$

### 2.2.4 Infill bars

$$\begin{aligned} \text{Max. span, } L &= 460 - 25 = 435 \text{ mm} \\ M_{Ed} &= 1.5 \cdot 0.5 \cdot 435 / 4 = 75.94 \text{ kN}\cdot\text{mm} \end{aligned}$$

#### Ø15mm / EN AW-6082 T6

$$\begin{aligned} W_{el} &= 0.33 \text{ cm}^3 \\ I &= 0.25 \text{ cm}^4 \\ M_{pl,Rd} &= 1.2 \cdot 0.33 \cdot 250 / 1.1 = 90.0 \text{ kN}\cdot\text{mm} \quad \underline{0.84 < 1.0} \\ \delta &\leq 435 / 65 = 6.69 \text{ mm} \\ \delta &= 500 \cdot 435^3 / (48 \cdot 70000 \cdot 2500) = 4.90 \text{ mm} \quad \underline{0.73 < 1.0} \end{aligned}$$

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### 2.3 Type Flat Balustrade

Refer to structural analysis results in section 2.3.3.

#### 2.3.1 Deflection check

$$\begin{aligned}\delta_{max} &= 4.4 \text{ mm} \\ \delta_{allow} = \min\{1100/65; 15\} &= 15.0 \text{ mm} \quad \underline{0.29 < 1.0}\end{aligned}$$

#### 2.3.2 Stress check to BS EN 1999-1-1

##### i Vertical members

$$\text{Max. Von mises stress, } \sigma_{max} = 58.2 \text{ N/mm}^2$$

##### EN AW-6082 T6

$$\sigma_{HAZ,Rd} = 0.5 \cdot 250 / 1.1 = 125.0 \text{ N/mm}^2 \quad \underline{0.46 < 1.0}$$

##### ii Horizontal members

$$\text{Max. Von mises stress, } \sigma_{max} = 38.9 \text{ N/mm}^2$$

$$\text{Max. HAZ stress, } \sigma_{HAZ} = 28.2 \text{ N/mm}^2$$

##### EN AW-6060 T6

$$\sigma_{el,Rd} = 140 / 1.1 = 127.27 \text{ N/mm}^2 \quad \underline{0.30 < 1.0}$$

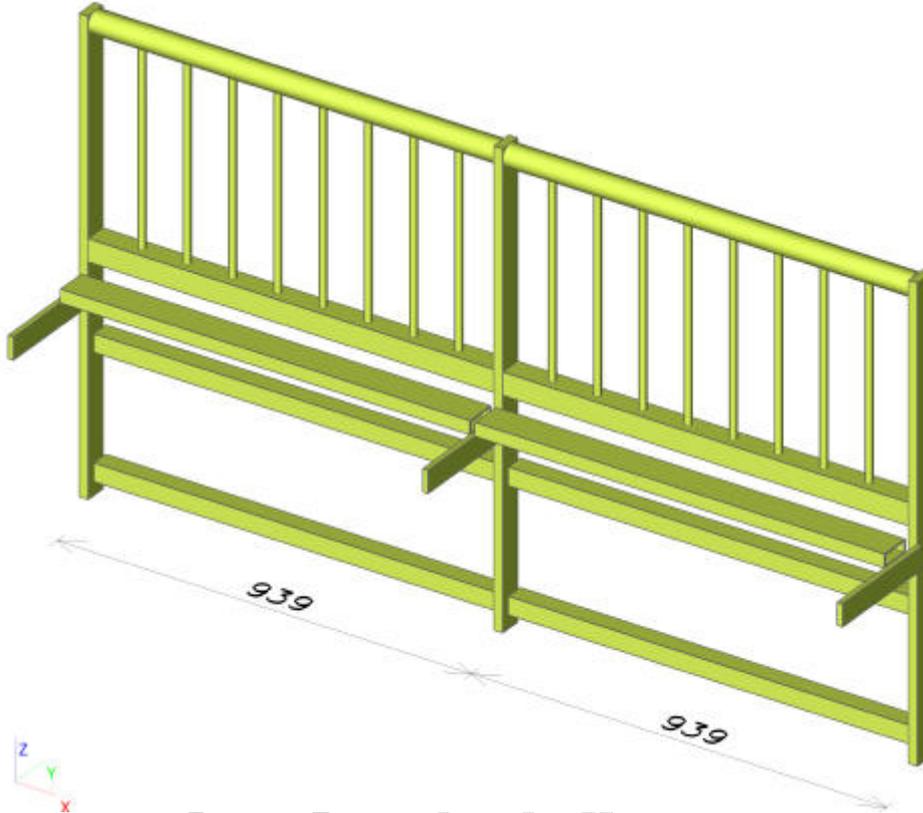
$$\sigma_{HAZ,Rd} = 0.43 \cdot 140 / 1.1 = 54.73 \text{ N/mm}^2 \quad \underline{0.52 < 1.0}$$

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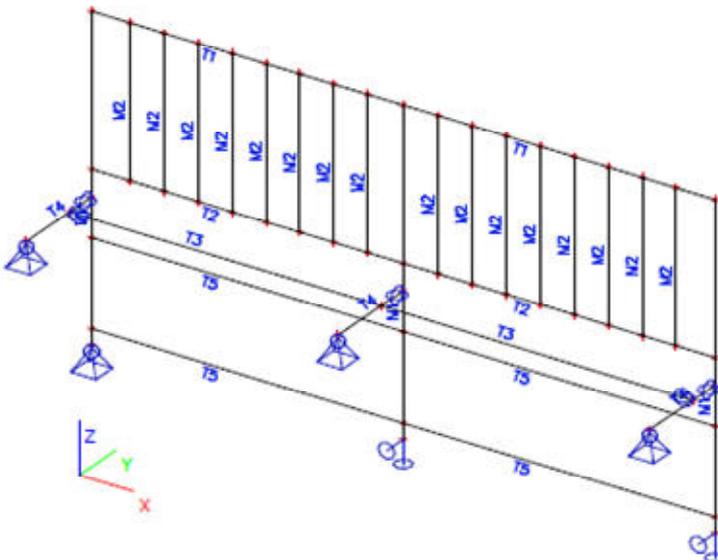
2.3.3 Structural analysis – Type flat balustrade

	Project	The Corniche, London
	Part	BAL-102 Type Flat
	Description	Aluminium - Welded assembly
	National code	EC - EN

1. Structural model



1.1. Analysis model

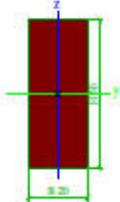


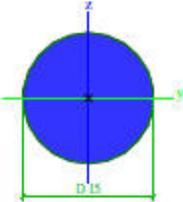
	Project	The Corniche, London
	Part	BAL-102 Type Flat
	Description	Aluminium - Welded assembly
	National code	EC - EN

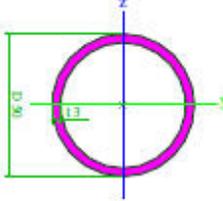
## 1.2. Materials

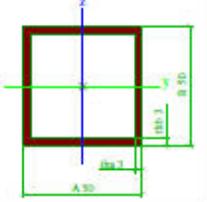
Name	Type	Unit mass [kg/m <sup>3</sup> ]	E mod [MPa]	Poisson - nu	G mod [MPa]	Thermal exp [m/mK]
EN-AW 6060 T6	Aluminium	2700.00	7.0000e+04	0.3	2.6923e+04	0.00
EN-AW 6082 T6	Aluminium	2700.00	7.0000e+04	0.3	2.6923e+04	0.00

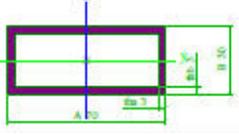
## 1.3. Cross-sections

	Name	M1	A [cm <sup>2</sup> ]	10.00	
	Type	Rectangle	A y, z [cm <sup>2</sup> ]	8.33	8.33
	Detailed	50; 20	I y, z [cm <sup>4</sup> ]	20.83	3.33
	Item material	EN-AW 6082 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	3.59	9.95
	Fabrication	general	Wet y, z [cm <sup>2</sup> ]	8.33	3.33
	Use 2D FEM analysis	<input checked="" type="checkbox"/>	Wpl y, z [cm <sup>2</sup> ]	12.50	5.00
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	10	25
		α [deg]	0.00		

	Name	M2	A [cm <sup>2</sup> ]	1.77	
	Type	Circle	A y, z [cm <sup>2</sup> ]	1.59	1.59
	Detailed	15	I y, z [cm <sup>4</sup> ]	0.25	0.25
	Item material	EN-AW 6082 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	0.00	0.50
	Fabrication	general	Wet y, z [cm <sup>2</sup> ]	0.33	0.33
	Use 2D FEM analysis	<input checked="" type="checkbox"/>	Wpl y, z [cm <sup>2</sup> ]	0.56	0.56
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	7	8
		α [deg]	0.00		

	Name	T1	A [cm <sup>2</sup> ]	4.43	
	Type	Tube	A y, z [cm <sup>2</sup> ]	2.97	2.97
	Detailed	50; 3	I y, z [cm <sup>4</sup> ]	12.28	12.28
	Item material	EN-AW 6060 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	0.00	23.96
	Fabrication	extrusion	Wet y, z [cm <sup>2</sup> ]	4.91	4.91
	Use 2D FEM analysis	<input checked="" type="checkbox"/>	Wpl y, z [cm <sup>2</sup> ]	6.64	6.64
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	25	25
		α [deg]	0.00		

	Name	T2	A [cm <sup>2</sup> ]	5.64	
	Type	O	A y, z [cm <sup>2</sup> ]	2.90	2.90
	Detailed	50; 3; 50; 3	I y, z [cm <sup>4</sup> ]	20.85	20.85
	Item material	EN-AW 6060 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	0.09	30.69
	Fabrication	extrusion	Wet y, z [cm <sup>2</sup> ]	8.34	8.34
	Use 2D FEM analysis	<input checked="" type="checkbox"/>	Wpl y, z [cm <sup>2</sup> ]	9.95	9.95
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	25	25
		α [deg]	0.00		

	Name	T3	A [cm <sup>2</sup> ]	5.64	
	Type	O	A y, z [cm <sup>2</sup> ]	3.92	1.78
	Detailed	70; 3; 30; 3	I y, z [cm <sup>4</sup> ]	8.38	33.32
	Item material	EN-AW 6060 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	7.98	20.67
	Fabrication	extrusion	Wet y, z [cm <sup>2</sup> ]	5.58	9.52
	Use 2D FEM analysis	<input checked="" type="checkbox"/>	Wpl y, z [cm <sup>2</sup> ]	6.53	12.17
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	35	15
		α [deg]	0.00		

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	Part	BAL-102 Type Flat
	Description	Aluminium - Welded assembly
	National code	EC - EN

	Name	T4	A [cm²]	6.00	
	Type	Rectangle	A y, z [cm²]	5.00	5.00
	Detailed	50; 12	I y, z [cm⁴]	12.50	0.72
	Item material	EN-AW 6060 T6	I w [cm²], t [cm²]	1.16	2.44
	Fabrication	general	Wei y, z [cm²]	5.00	1.20
	Use 2D FEM analysis	<input checked="" type="checkbox"/>	Wpl y, z [cm²]	7.50	1.80
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	6	25
			α [deg]	0.00	

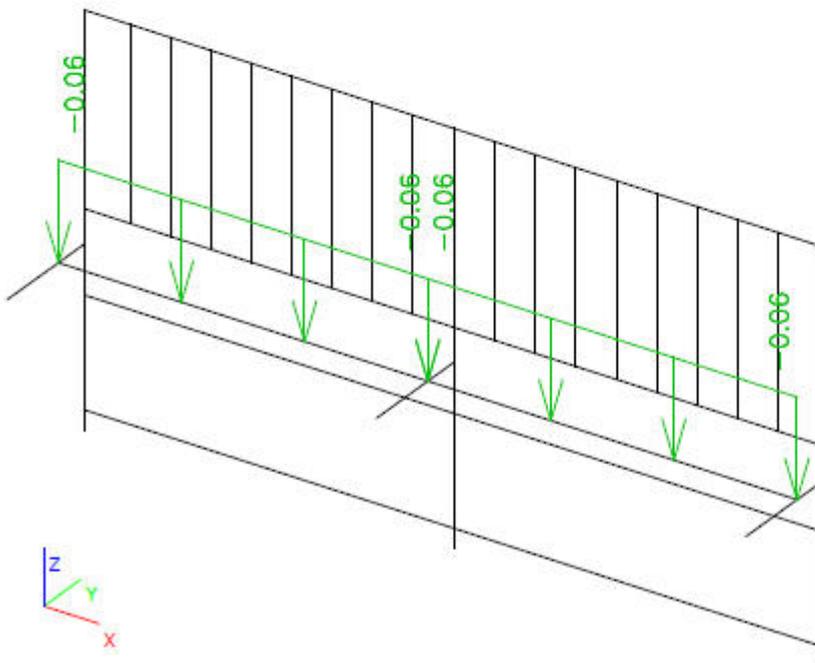
	Name	T5	A [cm²]	4.44	
	Type	O	A y, z [cm²]	2.31	2.31
	Detailed	40; 3; 40; 3	I y, z [cm⁴]	10.20	10.20
	Item material	EN-AW 6060 T6	I w [cm²], t [cm²]	0.04	15.12
	Fabrication	extrusion	Wei y, z [cm²]	5.10	5.10
	Use 2D FEM analysis	<input checked="" type="checkbox"/>	Wpl y, z [cm²]	6.17	6.17
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	20	20
			α [deg]	0.00	

Name	Mass [kg]	Surface [m²]	Volume [cm³]
Total results :	25.90	2.553	9593.56

Material	Mass [kg]	Surface [m²]	Unit volume mass [kg/m³]	Volume [cm³]
EN-AW 6060 T6	13.90	1.756	2700.00	5147.54
EN-AW 6062 T6	12.00	0.796	2700.00	4446.03

## 2. Loads [kN, kN/m]

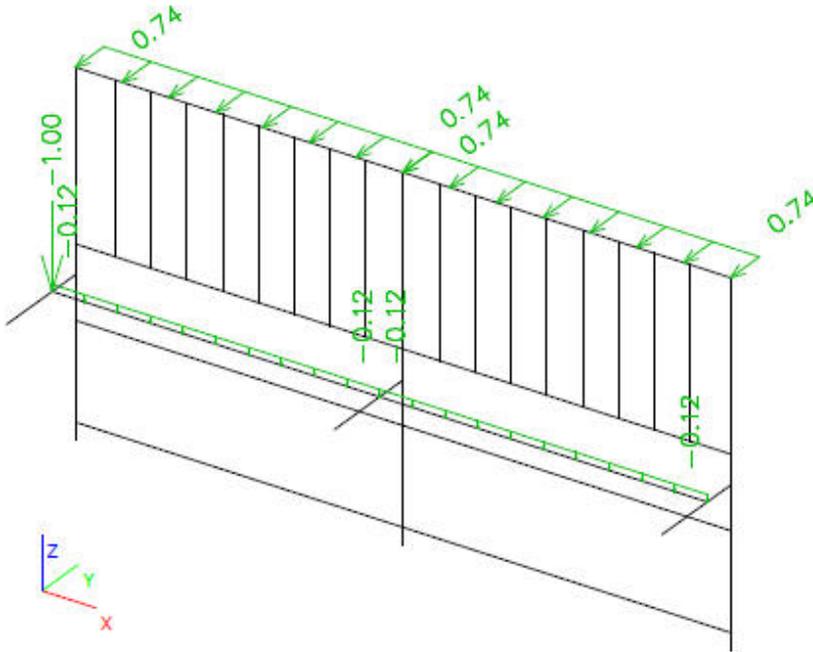
### 2.1. LC1 - D: Dead load



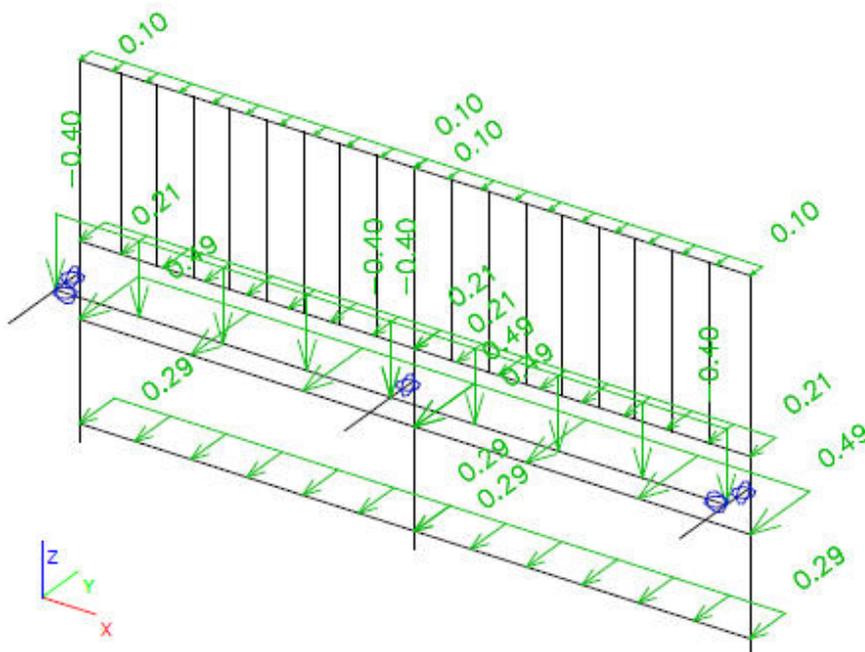
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	Project	The Corniche, London
	Part	BAL-102 Type Flat
	Description	Aluminium - Welded assembly
	National code	EC - EN

### 2.2. LC2 - I: Imposed load



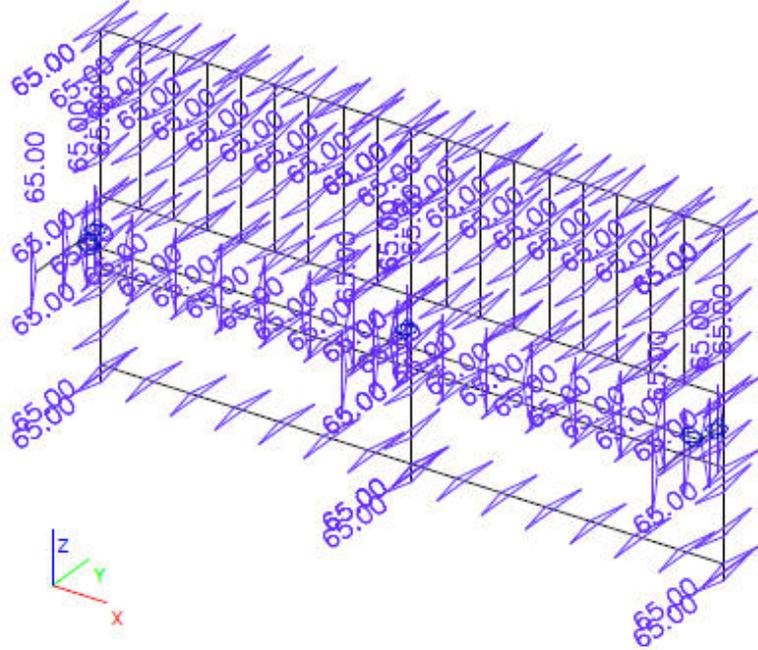
### 2.3. LC3 - W: Wind load



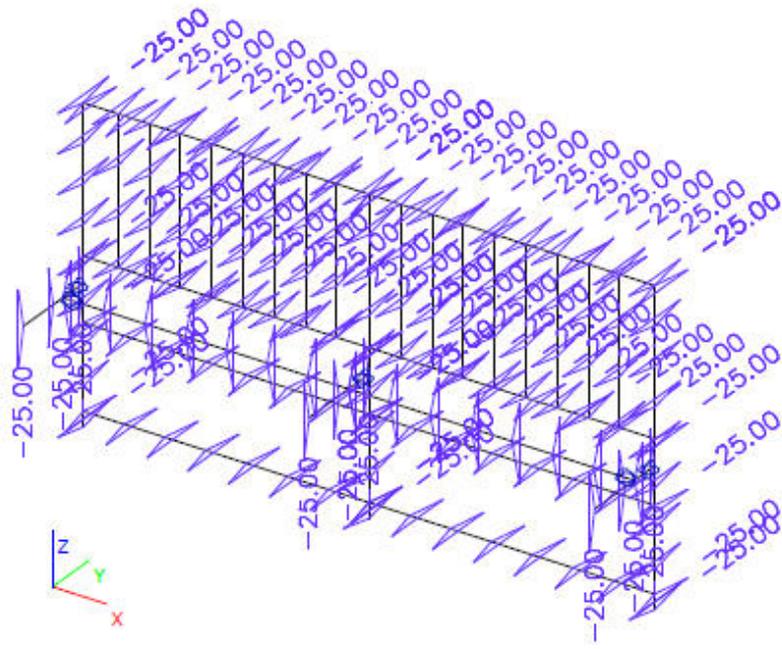
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	Project	The Corniche, London
	Part	BAL-102 Type Flat
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**2.4. LC4 - T1: Thermal load - Summer**



**2.5. LC5 - T2: Thermal load - Winter**



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	Part	BAL-102 Type Flat
	Description	Aluminium - Welded assembly
	National code	EC - EN

## 2.6. Nonlinear combinations

Name	Description	Type	Load cases	Coeff. [-]
NC101	(D) + (L)	Serviceability	LC0 - Selfweight	1.00
			LC1 - D - Dead load	1.00
			LC2 - L - Imposed/Live Load	1.00
NC102	(D) + (Wp)	Serviceability	LC0 - Selfweight	1.00
			LC1 - D - Dead load	1.00
			LC3 - W - Wind load Load	-1.00
NC103	(D) + (Ws)	Serviceability	LC0 - Selfweight	1.00
			LC1 - D - Dead load	1.00
			LC3 - W - Wind load Load	1.00
NC211	1.35(D) + 1.5(L) + 0.75(Wp) + 0.75(Ts)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC2 - L - Imposed/Live Load	1.50
			LC3 - W - Wind load Load	-0.75
			LC4 - T1 - Temperature Load, Summer	0.75
NC212	1.35(D) + 1.5(L) + 0.75(Ws) + 0.75(Ts)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC2 - L - Imposed/Live Load	1.50
			LC3 - W - Wind load Load	0.75
			LC4 - T1 - Temperature Load, Summer	0.75
NC213	1.35(D) + 1.5(Wp) + 0.9(Ts)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC3 - W - Wind load Load	-1.50
			LC4 - T1 - Temperature Load, Summer	0.90
NC214	1.35(D) + 1.5(Ws) + 0.9(Ts)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC3 - W - Wind load Load	1.50
			LC4 - T1 - Temperature Load, Summer	0.90
NC221	1.35(D) + 1.5(L) + 0.75(Wp) + 0.75(Tw)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC2 - L - Imposed/Live Load	1.50
			LC3 - W - Wind load Load	-0.75
			LC5 - T2 - Temperature Load, Winter	0.75
NC222	1.35(D) + 1.5(L) + 0.75(Ws) + 0.75(Tw)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC2 - L - Imposed/Live Load	1.50
			LC3 - W - Wind load Load	0.75
			LC5 - T2 - Temperature Load, Winter	0.75
NC223	1.35(D) + 1.5(Wp) + 0.9(Tw)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC3 - W - Wind load Load	-1.50
			LC5 - T2 - Temperature Load, Winter	0.90
NC224	1.35(D) + 1.5(Ws) + 0.9(Tw)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC3 - W - Wind load Load	1.50
			LC5 - T2 - Temperature Load, Winter	0.90
NC231	1.35(D) + 1.5(Ts)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC4 - T1 - Temperature Load, Summer	1.50
NC232	1.35(D) + 1.5(Tw)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC5 - T2 - Temperature Load, Winter	1.50

## 2.7. Result classes

Name	List
Serviceability	NC101
	NC102
	NC103
Ultimate	NC211
	NC212
	NC213
	NC214

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	Project	The Corniche, London
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	National code	EC - EN

Name	List
Ultimale	NC221
	NC222
	NC223
	NC224
	NC231
	NC232

### 3. Calculation protocol

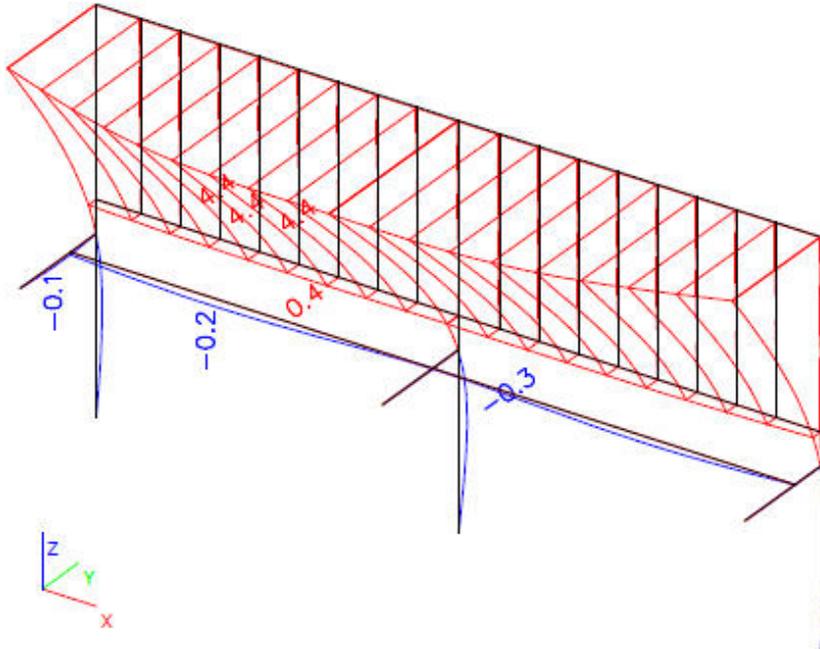
Calculation protocol				
<b>Nonlinear calculation</b>				
Number of 2D elements	0			
Number of 1D elements	157			
Number of mesh nodes	134			
Number of equations	804			
Maximum iterations	50			
Number of increments	2			
Type of nonlinearity	II. order			
Method (II. order)	local nonlinearities			
Bending theory	Newton-Raphson			
	Mindlin			
<b>No. of combination</b>	<b>Start</b>	<b>End</b>	<b>No. of iterations</b>	
NC 1	26.05.2015 14:02	26.05.2015 14:02	4	
NC 2	26.05.2015 14:02	26.05.2015 14:02	6	
NC 3	26.05.2015 14:02	26.05.2015 14:02	4	
NC 4	26.05.2015 14:02	26.05.2015 14:02	3	
NC 5	26.05.2015 14:02	26.05.2015 14:02	6	
NC 6	26.05.2015 14:02	26.05.2015 14:02	5	
NC 7	26.05.2015 14:02	26.05.2015 14:02	5	
NC 8	26.05.2015 14:02	26.05.2015 14:02	5	
NC 9	26.05.2015 14:02	26.05.2015 14:02	5	
NC 10	26.05.2015 14:02	26.05.2015 14:02	5	
NC 11	26.05.2015 14:02	26.05.2015 14:02	5	
NC 12	26.05.2015 14:02	26.05.2015 14:02	5	
NC 13	26.05.2015 14:02	26.05.2015 14:02	5	
<b>Sum of loads and reactions.</b>				
	<b>[kN]</b>	<b>X</b>	<b>Y</b>	<b>Z</b>
Nonlinear combination 1	loads	0.0	-1.4	-1.6
	reactions in nodes	0.0	1.4	1.6
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 2	loads	-0.0	-0.5	-1.8
	reactions in nodes	0.0	0.5	1.8
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 3	loads	0.0	2.0	0.4
	reactions in nodes	-0.0	-2.0	-0.4
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 4	loads	-0.0	-2.0	-1.1
	reactions in nodes	0.0	2.0	1.1
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 5	loads	-0.0	-3.6	-2.9
	reactions in nodes	0.0	3.6	2.9
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 6	loads	0.0	3.1	0.6
	reactions in nodes	-0.0	-3.1	-0.6
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 7	loads	-0.0	-3.1	-1.6
	reactions in nodes	0.0	3.1	1.6
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 8	loads	-0.0	-0.5	-1.8

	Project	The Corniche, London
	Part	BAL-102 Type Flat
	Description	Aluminium - Welded assembly
	National code	EC - EN

Calculation protocol					
	kN	X	Y	Z	
	reactions in nodes	0.0	0.5	1.8	
	reactions on lines	0.0	0.0	0.0	
	contact 1D	0.0	0.0	0.0	
	contact 2D	0.0	0.0	0.0	
Nonlinear combination 9	loads	-0.0	-3.6	-2.9	
	reactions in nodes	0.0	3.6	2.9	
	reactions on lines	0.0	0.0	0.0	
	contact 1D	0.0	0.0	0.0	
	contact 2D	0.0	0.0	0.0	
Nonlinear combination 10	loads	0.0	3.1	0.6	
	reactions in nodes	-0.0	-3.1	-0.6	
	reactions on lines	0.0	0.0	0.0	
	contact 1D	0.0	0.0	0.0	
	contact 2D	0.0	0.0	0.0	
Nonlinear combination 11	loads	-0.0	-3.1	-1.6	
	reactions in nodes	0.0	3.1	1.6	
	reactions on lines	0.0	0.0	0.0	
	contact 1D	0.0	0.0	0.0	
	contact 2D	0.0	0.0	0.0	
Nonlinear combination 12	loads	0.0	0.0	-0.5	
	reactions in nodes	0.0	-0.0	0.5	
	reactions on lines	0.0	0.0	0.0	
	contact 1D	0.0	0.0	0.0	
	contact 2D	0.0	0.0	0.0	
Nonlinear combination 13	loads	0.0	0.0	-0.5	
	reactions in nodes	0.0	-0.0	0.5	
	reactions on lines	0.0	0.0	0.0	
	contact 1D	0.0	0.0	0.0	
	contact 2D	0.0	0.0	0.0	

#### 4. Deflection

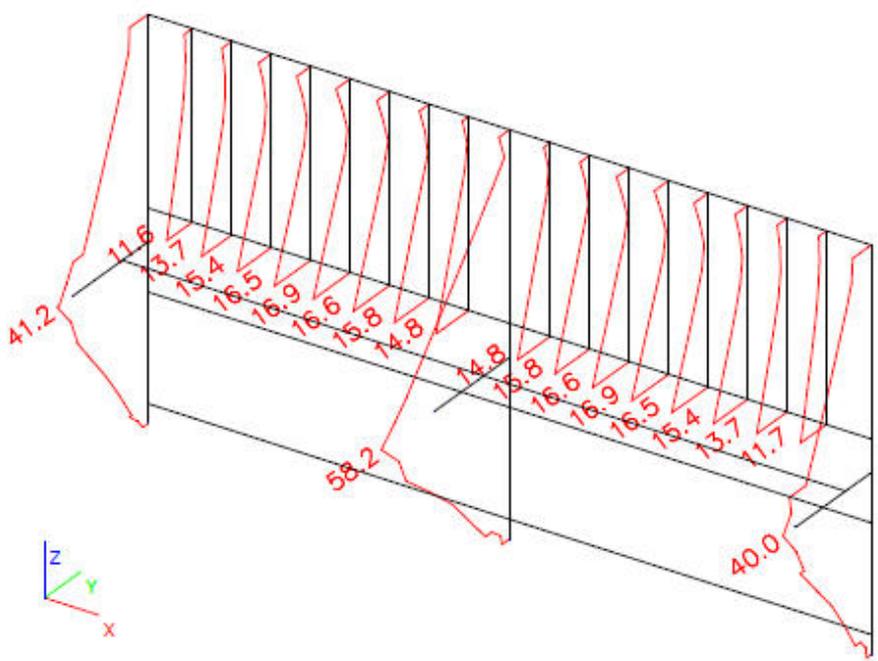
##### 4.1. Deformations on member; uz: NC101



	Project	The Corniche, London
	Part	BAL-102 Type Flat
	Description	Aluminium - Welded assembly
	National code	EC - EN

**5. Member code check**

**5.1. Stress; von Mises: Vertical members**



Nonlinear calculation, Extreme : Global  
 Selection : All  
 Nonlinear combinations : NC211  
 Cross-section : M1 - Rectangle (50; 20)  
 Values : Normal -, Normal +, Shear, von Mises, Fatigue, Kappa, Sigma Y

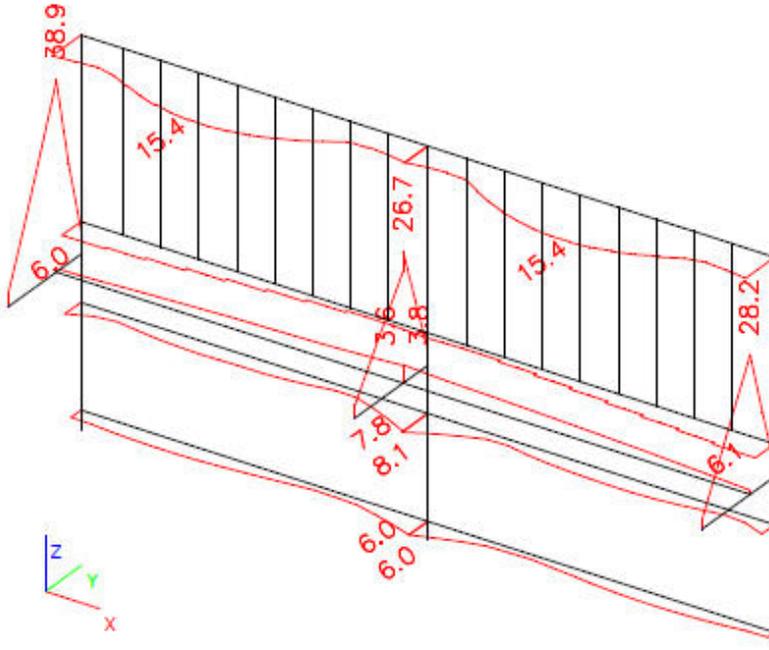
Member	Case	dx [m]	Normal - [MPa]	Normal + [MPa]	Shear [MPa]	von Mises [MPa]	Fatigue [MPa]	Kappa [-]
B120	NC211	0.455	-58.2		0.0	58.2		
B120	NC211	0.455	58.0	58.0			0.0	1.00
B120	NC211	1.025	-4.3	-4.3			0.0	1.00
B120	NC211	0.455		58.0	0.0	58.0		
B99	NC211	0.000	-1.1	-1.1	0.0	1.1		
B99	NC211	0.782		0.0	4.9	8.4		
B99	NC211	0.000	-1.1	-1.1			0.0	1.00

Nonlinear calculation, Extreme : Global  
 Selection : All  
 Nonlinear combinations : NC211  
 Cross-section : M2 - Circle (15)  
 Values : Normal -, Normal +, Shear, von Mises, Fatigue, Kappa, Sigma Y

Member	Case	dx [m]	Normal - [MPa]	Normal + [MPa]	Shear [MPa]	von Mises [MPa]	Fatigue [MPa]	Kappa [-]
B114	NC211	0.000	-16.8		0.6	16.9		
B107	NC211	0.000	16.8	16.8			0.0	1.00
B114	NC211	0.485	-6.3	-6.3			0.0	1.00
B107	NC211	0.000		16.8	0.6	16.8		
B103	NC211	0.000	11.2	11.2			0.0	1.00
B103	NC211	0.061		4.7	1.8	5.6		
B103	NC211	0.000	11.2	11.2	1.7	11.5		

	Project	The Corniche, London
	Part	BAL-102 Type Flat
	Description	Aluminium - Welded assembly
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### 5.2. Stress; von Mises: Horizontal members



Nonlinear calculation, Extreme : Global  
 Selection : All  
 Nonlinear combinations : NC211  
 Cross-section : T1 - Tube (50; 3)  
 Values : Normal -, Normal +, Shear, von Mises, Fatigue, Kappa, Sigma Y

Member	Case	dx [m]	Normal - [MPa]	Normal + [MPa]	Shear [MPa]	von Mises [MPa]	Fatigue [MPa]	Kappa [-]
B132	NC211	0.464	-14.6		2.7	15.3		
B130	NC211	0.474	14.6	14.6			0.0	1.00
B132	NC211	0.000	-6.3	-6.3			0.0	1.00
B130	NC211	0.474		14.6	2.7	15.3		
B130	NC211	0.000	-4.4	-4.4			0.0	1.00
B130	NC211	0.000	-0.9		5.2	9.1		
B132	NC211	0.515	-14.6		2.9	15.4		
B130	NC211	0.000	-4.4	-4.4	3.3	7.2		

Nonlinear calculation, Extreme : Global  
 Selection : All  
 Nonlinear combinations : NC211  
 Cross-section : T2 - O (50; 3; 50; 3)  
 Values : Normal -, Normal +, Shear, von Mises, Fatigue, Kappa, Sigma Y

Member	Case	dx [m]	Normal - [MPa]	Normal + [MPa]	Shear [MPa]	von Mises [MPa]	Fatigue [MPa]	Kappa [-]
B129	NC211	0.939	-1.6		0.0	1.6		
B129	NC211	0.000	0.4	0.4			0.0	1.00
B129	NC211	0.939	-1.6	-1.6			0.0	1.00
B129	NC211	0.939		2.5	0.0	2.5		
B97	NC211	0.000	-0.7	-0.7			0.0	1.00
B97	NC211	0.000		0.3	3.4	6.0		
B129	NC211	0.939		1.5	3.4	6.1		
B97	NC211	0.000	-0.7	-0.7	0.0	0.7		

Nonlinear calculation, Extreme : Global  
 Selection : All  
 Nonlinear combinations : NC211  
 Cross-section : T3 - O (70; 3; 30; 3)  
 Values : Normal -, Normal +, Shear, von Mises, Fatigue, Kappa, Sigma Y

	Project	The Corniche, London
	Part	BAL-102 Type Flat
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Member	Case	dx [m]	Normal - [MPa]	Normal + [MPa]	Shear [MPa]	von Mises [MPa]	Fatigue [MPa]	Kappa [-]
B128	NC211	0.000	-3.8		0.0	3.8		
B98	NC211	0.939	2.5	2.5			0.0	1.00
B128	NC211	0.000	-3.4	-3.4			0.0	1.00
B98	NC211	0.939	2.5		0.0	2.5		
B98	NC211	0.000	-0.6	-0.6			0.0	1.00
B98	NC211	0.000	-0.6		0.3	0.7		
B98	NC211	0.000	-0.6	-0.6	0.0	0.6		

Nonlinear calculation, Extreme : Global

Selection : All

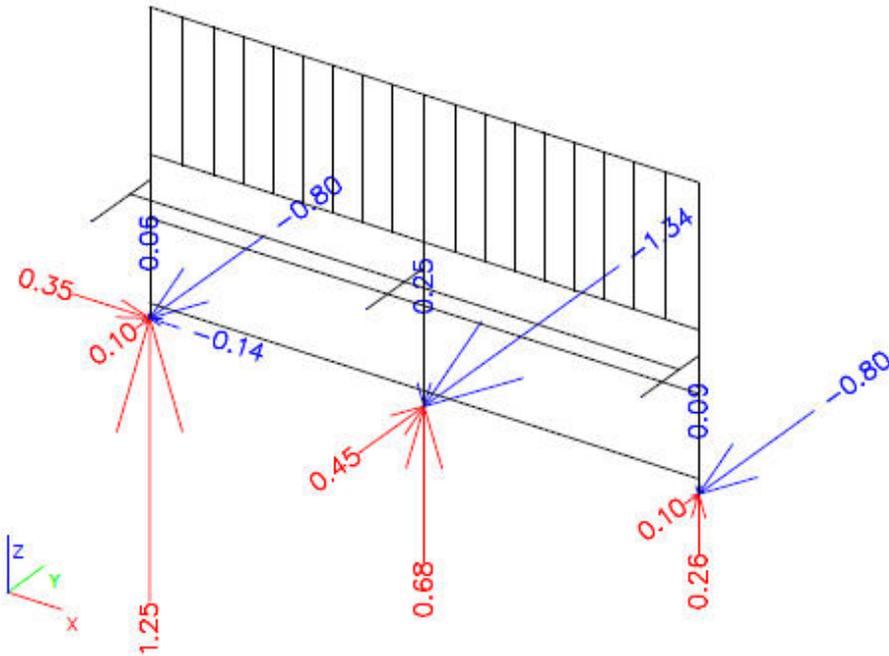
Nonlinear combinations : NC211

Cross-section : T4 - Rectangle (50; 12)

Values : Normal -, Normal +, Shear, von Mises, Fatigue, Kappa, Sigma Y

Member	Case	dx [m]	Normal - [MPa]	Normal + [MPa]	Shear [MPa]	von Mises [MPa]	Fatigue [MPa]	Kappa [-]
B100	NC211	0.100	-38.9		0.0	38.9		
B102	NC211	0.100	24.8	24.8			0.0	1.00
B101	NC211	0.100	-26.7	-26.7			0.0	1.00
B100	NC211	0.100	35.7		0.0	35.7		
B100	NC211	0.000	-1.6	-1.6			0.0	1.00
B100	NC211	0.000	-1.6		2.7	4.9		
B100	NC211	0.000	-1.6	-1.6	0.0	1.6		

## 6. Factored reaction forces



Nonlinear calculation, Extreme : Global

Selection : Named selection - Support - base

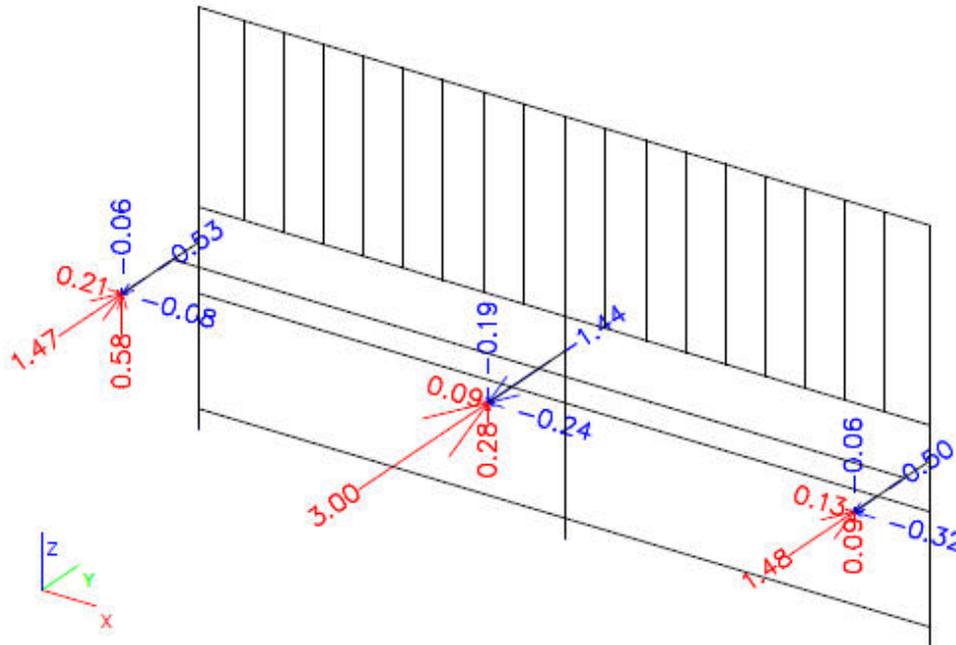
Class : Ultimate

Rotated supports

Support	Case	Rx [kN]	Ry [kN]	Rz [kN]	Mx [kNcm]	My [kNcm]	Mz [kNcm]
Sn1/N192	NC232	-0.14	0.00	0.07	0.00	0.00	0.00
Sn1/N192	NC231	0.35	0.00	0.19	0.00	0.00	0.00
Sn2/N234	NC211	0.00	-1.34	0.13	0.00	0.00	0.00
Sn2/N234	NC214	0.00	0.45	0.68	0.00	0.00	0.00

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	Part	BAL-102 Type Flat
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Support	Case	Rx [kN]	Ry [kN]	Rz [kN]	Mx [kNcm]	My [kNcm]	Mz [kNcm]
Sn2/N234	NC223	0.00	-0.45	<b>-0.25</b>	0.00	0.00	0.00
Sn1/N192	NC212	0.16	-0.70	<b>1.25</b>	0.00	0.00	0.00
Sn1/N192	NC211	0.17	-0.80	1.11	<b>0.00</b>	<b>0.00</b>	<b>0.00</b>



Nonlinear calculation, Extreme : Global  
 Selection : Named selection - Support - strut  
 Class : Ultimate  
 Rotated supports

Support	Case	Rx [kN]	Ry [kN]	Rz [kN]	Mx [kNcm]	My [kNcm]	Mz [kNcm]
Sn15/N199	NC231	<b>-0.32</b>	0.05	0.02	0.00	0.00	0.00
Sn13/N195	NC231	<b>0.21</b>	-0.06	0.02	0.00	0.00	0.00
Sn14/N197	NC213	-0.15	<b>-1.44</b>	<b>-0.19</b>	0.00	0.00	0.00
Sn14/N197	NC212	-0.11	<b>3.00</b>	0.24	0.00	0.00	0.00
Sn13/N195	NC212	0.10	1.43	<b>0.58</b>	0.00	0.00	0.00
Sn13/N195	NC211	0.10	0.94	0.50	<b>0.00</b>	<b>0.00</b>	<b>0.00</b>

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## 2.4 Type J Balustrade

Refer to structural analysis results in section 2.4.3.

### 2.4.1 Deflection check

$$\begin{aligned}\delta_{max} &= 3.9 \text{ mm} \\ \delta_{allow} = \min\{1100/65; 15\} &= 15.0 \text{ mm} \quad \underline{0.26 < 1.0}\end{aligned}$$

### 2.4.2 Stress check to BS EN 1999-1-1

#### i Vertical members

$$\text{Max. Von mises stress, } \sigma_{max} = 58.9 \text{ N/mm}^2$$

#### EN AW-6082 T6

$$\sigma_{HAZ,Rd} = 0.5 \cdot 250 / 1.1 = 125.0 \text{ N/mm}^2 \quad \underline{0.47 < 1.0}$$

#### ii Horizontal members

$$\text{Max. Von mises stress, } \sigma_{max} = 65.4 \text{ N/mm}^2$$

$$\text{Max. HAZ stress, } \sigma_{HAZ} = 18.7 \text{ N/mm}^2$$

#### EN AW-6060 T6

$$\sigma_{el,Rd} = 140 / 1.1 = 127.27 \text{ N/mm}^2 \quad \underline{0.51 < 1.0}$$

$$\sigma_{HAZ,Rd} = 0.43 \cdot 140 / 1.1 = 54.73 \text{ N/mm}^2 \quad \underline{0.34 < 1.0}$$

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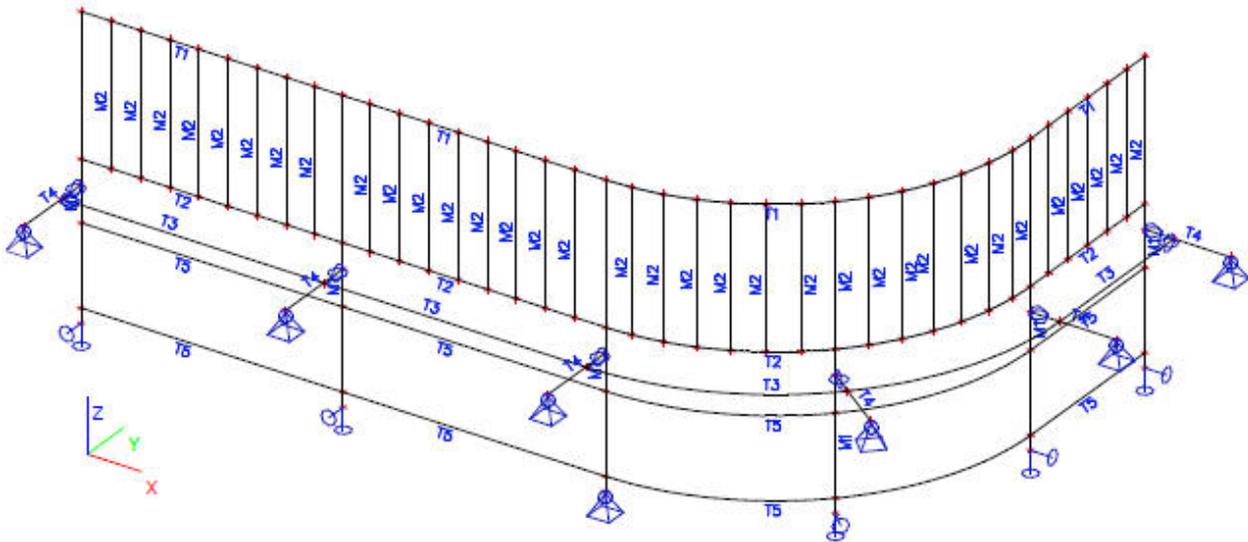
2.4.3 Structural analysis – Type J balustrade

	Project	The Corniche, London
	Part	BAL-102 Type J
	Description	Aluminium - Welded assembly
	National code	EC - EN

1. Structural model



1.1. Parameters

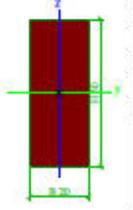


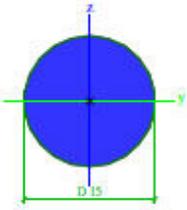
1.2. Materials

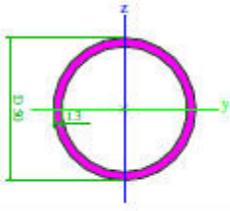
Name	Type	Unit mass [kg/m <sup>3</sup> ]	E mod [MPa]	Poisson - nu	G mod [MPa]	Thermal exp [m/mK]
EN-AW 6060 T6	Aluminium	2700.00	7.0000e+04	0.3	2.6923e+04	0.00
EN-AW 6082 T6	Aluminium	2700.00	7.0000e+04	0.3	2.6923e+04	0.00

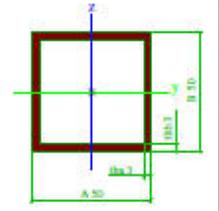
	Project	The Corniche, London
	Part	BAL-102 Type J
	Description	Aluminium - Welded assembly
	National code	EC - EN

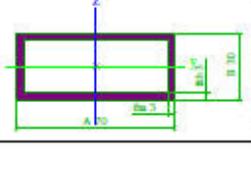
### 1.3. Cross-sections

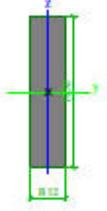
	Name	M1	A [cm <sup>2</sup> ]	10.00	
	Type	Rectangle	A y, z [cm <sup>2</sup> ]	8.33	8.33
	Detailed	50; 20	I y, z [cm <sup>4</sup> ]	20.83	3.33
	Item material	EN-AW 6082 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	3.59	9.95
	Fabrication	general	Wei y, z [cm <sup>2</sup> ]	8.33	3.33
	Use 2D FEM analysis	<input checked="" type="checkbox"/>	Wpl y, z [cm <sup>2</sup> ]	12.50	5.00
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	10	25
			α [deg]	0.00	

	Name	M2	A [cm <sup>2</sup> ]	1.77	
	Type	Circle	A y, z [cm <sup>2</sup> ]	1.59	1.59
	Detailed	15	I y, z [cm <sup>4</sup> ]	0.25	0.25
	Item material	EN-AW 6082 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	0.00	0.50
	Fabrication	general	Wei y, z [cm <sup>2</sup> ]	0.33	0.33
	Use 2D FEM analysis	<input checked="" type="checkbox"/>	Wpl y, z [cm <sup>2</sup> ]	0.56	0.56
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	7	8
			α [deg]	0.00	

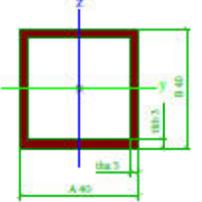
	Name	T1	A [cm <sup>2</sup> ]	4.43	
	Type	Tube	A y, z [cm <sup>2</sup> ]	2.97	2.97
	Detailed	50; 3	I y, z [cm <sup>4</sup> ]	12.28	12.28
	Item material	EN-AW 6060 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	0.00	23.96
	Fabrication	extrusion	Wei y, z [cm <sup>2</sup> ]	4.91	4.91
	Use 2D FEM analysis	<input checked="" type="checkbox"/>	Wpl y, z [cm <sup>2</sup> ]	6.64	6.64
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	25	25
			α [deg]	0.00	

	Name	T2	A [cm <sup>2</sup> ]	5.64	
	Type	O	A y, z [cm <sup>2</sup> ]	2.90	2.90
	Detailed	50; 3; 50; 3	I y, z [cm <sup>4</sup> ]	20.85	20.85
	Item material	EN-AW 6060 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	0.09	30.69
	Fabrication	extrusion	Wei y, z [cm <sup>2</sup> ]	8.34	8.34
	Use 2D FEM analysis	<input checked="" type="checkbox"/>	Wpl y, z [cm <sup>2</sup> ]	9.95	9.95
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	25	25
			α [deg]	0.00	

	Name	T3	A [cm <sup>2</sup> ]	5.64	
	Type	O	A y, z [cm <sup>2</sup> ]	3.92	1.78
	Detailed	70; 3; 30; 3	I y, z [cm <sup>4</sup> ]	8.38	33.32
	Item material	EN-AW 6060 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	7.98	20.67
	Fabrication	extrusion	Wei y, z [cm <sup>2</sup> ]	5.58	9.52
	Use 2D FEM analysis	<input checked="" type="checkbox"/>	Wpl y, z [cm <sup>2</sup> ]	6.53	12.17
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	35	15
			α [deg]	0.00	

	Name	T4	A [cm <sup>2</sup> ]	6.00	
	Type	Rectangle	A y, z [cm <sup>2</sup> ]	5.00	5.00
	Detailed	50; 12	I y, z [cm <sup>4</sup> ]	12.50	0.72
	Item material	EN-AW 6060 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	1.16	2.44
	Fabrication	general	Wei y, z [cm <sup>2</sup> ]	5.00	1.20
	Use 2D FEM analysis	<input checked="" type="checkbox"/>	Wpl y, z [cm <sup>2</sup> ]	7.50	1.80
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	8	25
			α [deg]	0.00	

	Project	The Corniche, London
	Part	BAL-102 Type J
	Description	Aluminium - Welded assembly
	National code	EC - EN

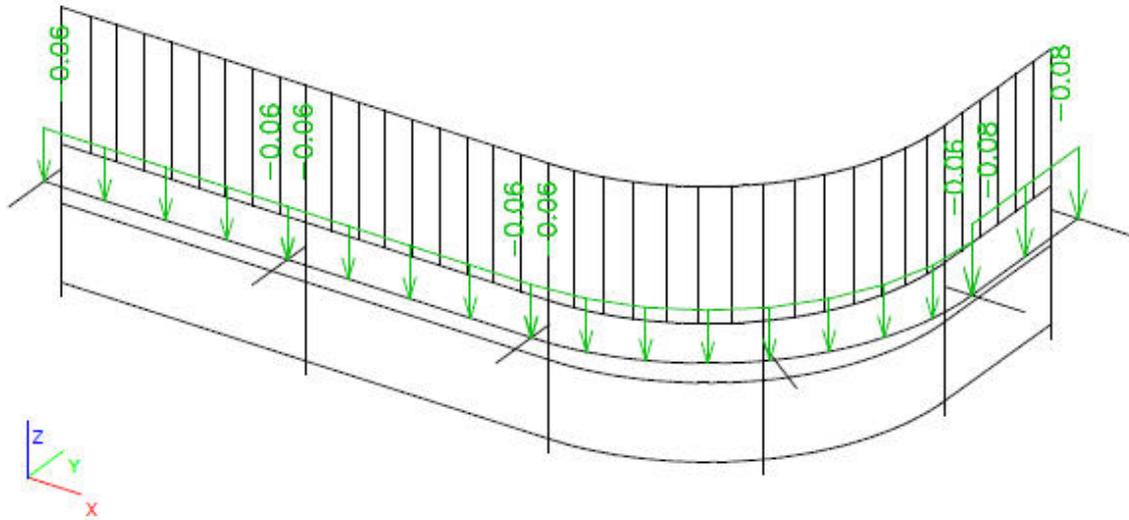
	Name	T5	A [cm²]	4.44
	Type	O	A y, z [cm²]	2.31 2.31
	Detailed	40; 3; 40; 3	I y, z [cm⁴]	10.20 10.20
	Item material	EN-AW 6060 T6	I w [cm⁴], t [cm²]	0.04 15.12
	Fabrication	extrusion	Wei y, z [cm²]	5.10 5.10
	Use 2D FEM analysis	✓	Wpl y, z [cm²]	6.17 6.17
			d y, z [mm]	0 0
			c YUCS, ZUCS [mm]	20 20
			α [deg]	0.00

Name	Mass [kg]	Surface [m²]	Volume [cm³]
Total results :	51.23	5.119	18974.20

Material	Mass [kg]	Surface [m²]	Unit volume mass [kg/m³]	Volume [cm³]
EN-AW 6060 T6	28.07	3.549	2700.00	10395.77
EN-AW 6082 T6	23.16	1.570	2700.00	8578.43

## 2. Loads [kN, kN/m]

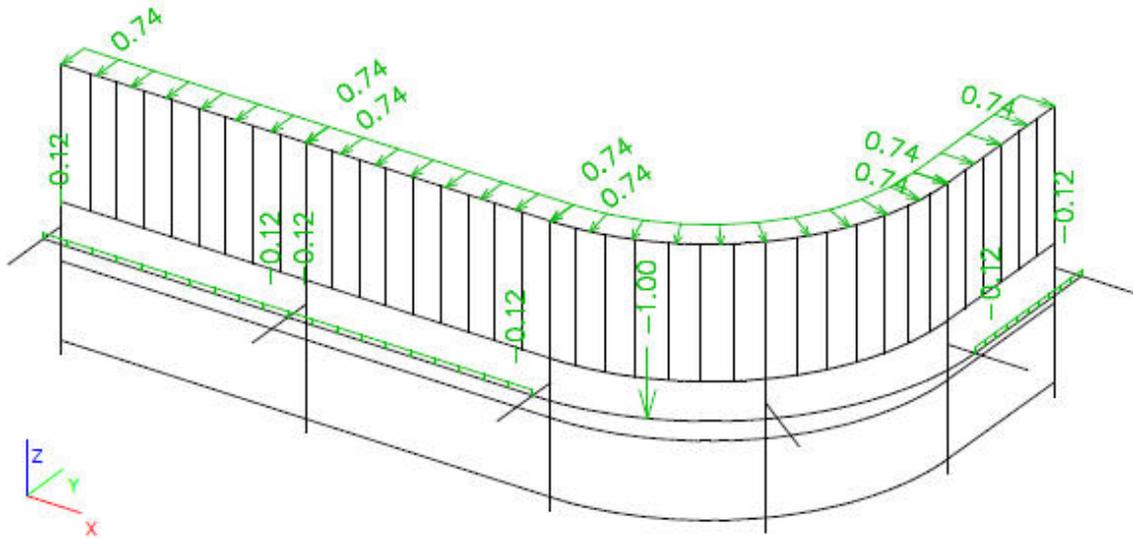
### 2.1. LC1 - D: Dead load



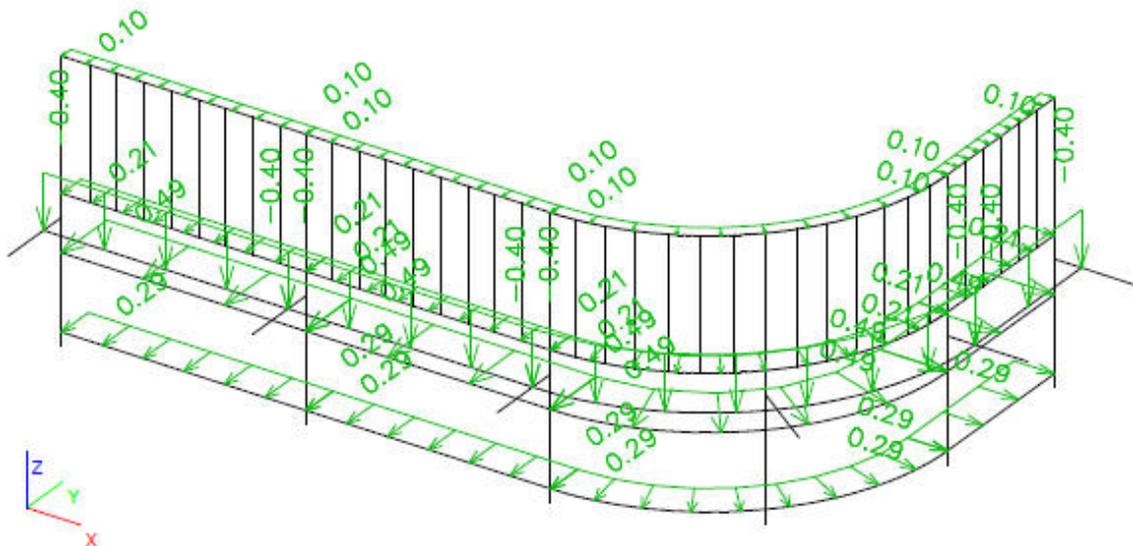
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**2.2. LC2 - I: Imposed load**



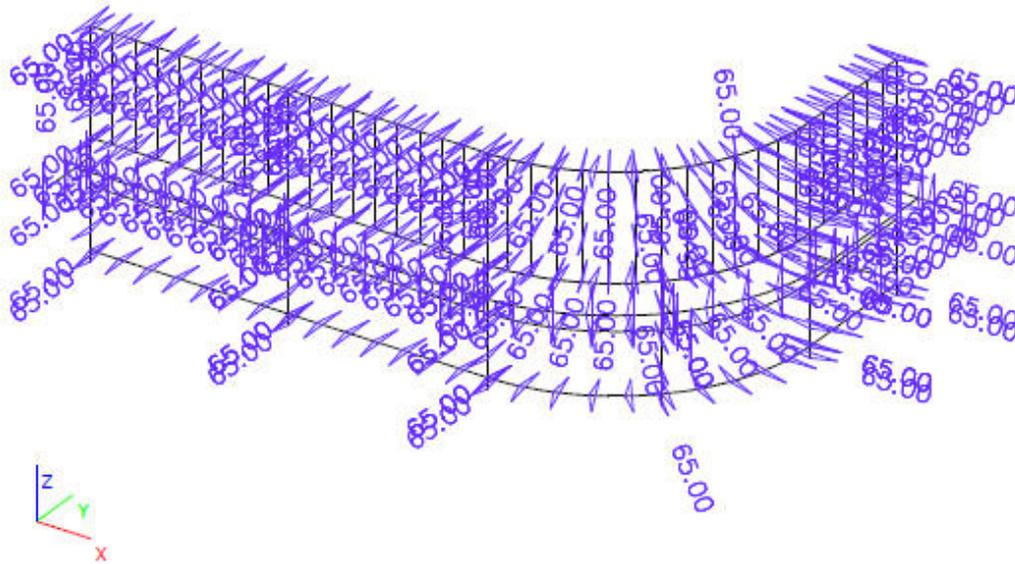
**2.3. LC3 - W: Wind load**



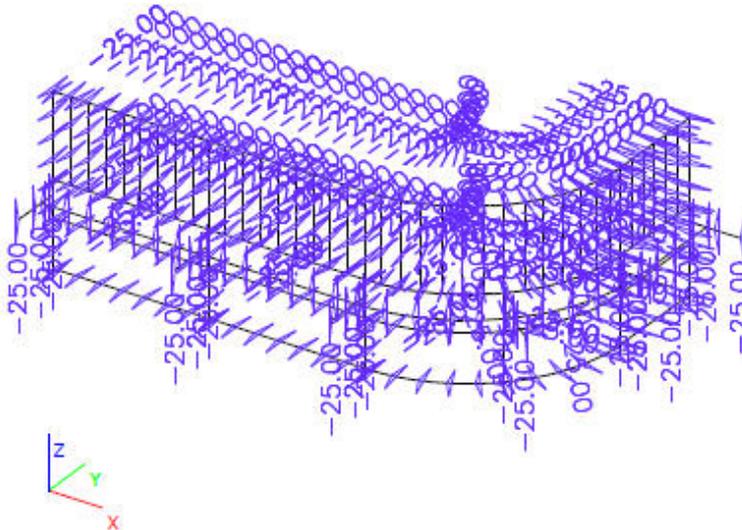
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**2.4. LC4 - T1: Thermal load - Summer**



**2.5. LC5 - T2: Thermal load - Winter**



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## 2.6. Nonlinear combinations

Name	Description	Type	Load cases	Coeff. [-]
NC101	(D) + (L)	Serviceability	LC0 - Selfweight	1.00
			LC1 - D - Dead load	1.00
			LC2 - L - Imposed/Live Load	1.00
NC102	(D) + (Wp)	Serviceability	LC0 - Selfweight	1.00
			LC1 - D - Dead load	1.00
			LC3 - W - Wind load Load	-1.00
NC103	(D) + (Ws)	Serviceability	LC0 - Selfweight	1.00
			LC1 - D - Dead load	1.00
			LC3 - W - Wind load Load	1.00
NC211	1.35(D) + 1.5(L) + 0.75(Wp) + 0.75(Ts)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC2 - L - Imposed/Live Load	1.50
			LC3 - W - Wind load Load	-0.75
NC212	1.35(D) + 1.5(L) + 0.75(Ws) + 0.75(Ts)	Ultimate	LC4 - T1 - Temperature Load, Summer	0.75
			LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC2 - L - Imposed/Live Load	1.50
			LC3 - W - Wind load Load	0.75
NC213	1.35(D) + 1.5(Wp) + 0.9(Ts)	Ultimate	LC4 - T1 - Temperature Load, Summer	0.75
			LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC3 - W - Wind load Load	-1.50
			LC4 - T1 - Temperature Load, Summer	0.90
NC214	1.35(D) + 1.5(Ws) + 0.9(Ts)	Ultimate	LC4 - T1 - Temperature Load, Summer	0.90
			LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC3 - W - Wind load Load	1.50
			LC4 - T1 - Temperature Load, Summer	0.90
NC221	1.35(D) + 1.5(L) + 0.75(Wp) + 0.75(Tw)	Ultimate	LC4 - T1 - Temperature Load, Summer	0.90
			LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC2 - L - Imposed/Live Load	1.50
			LC3 - W - Wind load Load	-0.75
NC222	1.35(D) + 1.5(L) + 0.75(Ws) + 0.75(Tw)	Ultimate	LC5 - T2 - Temperature Load, Winter	0.75
			LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC2 - L - Imposed/Live Load	1.50
			LC3 - W - Wind load Load	0.75
NC223	1.35(D) + 1.5(Wp) + 0.9(Tw)	Ultimate	LC5 - T2 - Temperature Load, Winter	0.75
			LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC3 - W - Wind load Load	-1.50
			LC5 - T2 - Temperature Load, Winter	0.90
NC224	1.35(D) + 1.5(Ws) + 0.9(Tw)	Ultimate	LC5 - T2 - Temperature Load, Winter	0.90
			LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC3 - W - Wind load Load	1.50
			LC5 - T2 - Temperature Load, Winter	0.90
NC231	1.35(D) + 1.5(Ts)	Ultimate	LC4 - T1 - Temperature Load, Summer	1.50
			LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC4 - T1 - Temperature Load, Summer	1.50
NC232	1.35(D) + 1.5(Tw)	Ultimate	LC5 - T2 - Temperature Load, Winter	1.50
			LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35

## 2.7. Result classes

Name	List
Serviceability	NC101
	NC102
	NC103
Ultimate	NC211
	NC212
	NC213
	NC214

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Name	List
Ultimate	NC221
	NC222
	NC223
	NC224
	NC231
	NC232

### 3. Calculation protocol

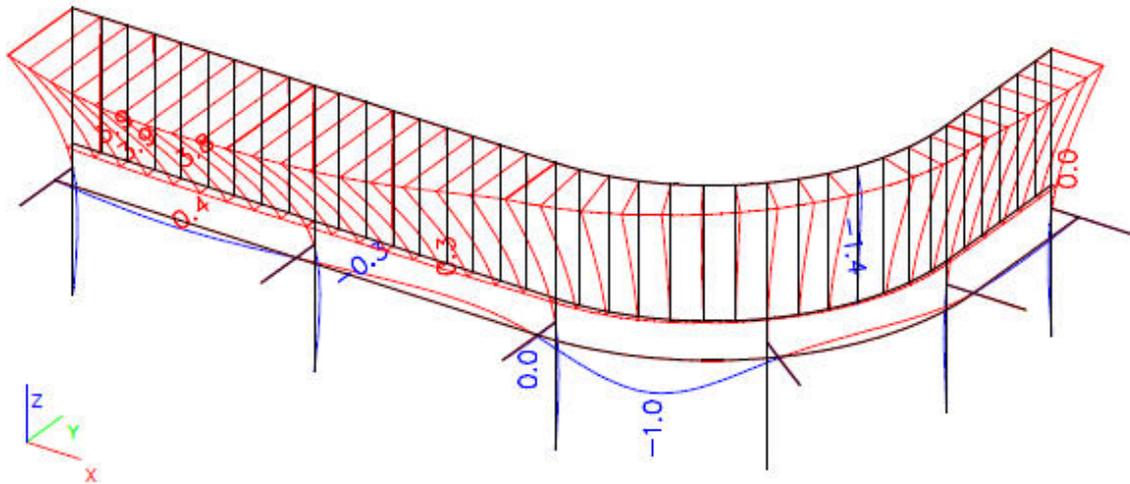
Calculation protocol				
<b>Nonlinear calculation</b>				
Number of 2D elements	0			
Number of 1D elements	542			
Number of mesh nodes	490			
Number of equations	2940			
Maximum iterations	50			
Number of increments	2			
Type of nonlinearity	II. order			
Method (II. order)	local nonlinearities			
Bending theory	Newton-Raphson			
	Mindlin			
No. of combination	Start	End	No. of iterations	
NC 1	26.05.2015 14:24	26.05.2015 14:24	4	
NC 2	26.05.2015 14:24	26.05.2015 14:24	5	
NC 3	26.05.2015 14:24	26.05.2015 14:24	4	
NC 4	26.05.2015 14:24	26.05.2015 14:24	4	
NC 5	26.05.2015 14:24	26.05.2015 14:24	5	
NC 6	26.05.2015 14:24	26.05.2015 14:24	5	
NC 7	26.05.2015 14:24	26.05.2015 14:24	5	
NC 8	26.05.2015 14:24	26.05.2015 14:24	5	
NC 9	26.05.2015 14:24	26.05.2015 14:24	5	
NC 10	26.05.2015 14:24	26.05.2015 14:24	5	
NC 11	26.05.2015 14:24	26.05.2015 14:24	5	
NC 12	26.05.2015 14:24	26.05.2015 14:24	5	
NC 13	26.05.2015 14:24	26.05.2015 14:24	5	
<b>Sum of loads and reactions.</b>				
	[kN]	X	Y	Z
Nonlinear combination 1	loads	1.1	-2.0	-2.0
	reactions in nodes	-1.1	2.0	2.0
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 2	loads	0.4	-0.8	-1.8
	reactions in nodes	-0.4	0.8	1.8
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 3	loads	-1.6	2.9	0.8
	reactions in nodes	1.6	-2.9	-0.8
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 4	loads	1.6	-2.9	-2.3
	reactions in nodes	-1.6	2.9	2.3
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 5	loads	2.8	-5.2	-4.2
	reactions in nodes	-2.8	5.2	4.2
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 6	loads	-2.4	4.4	1.3
	reactions in nodes	2.4	-4.4	-1.3
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 7	loads	2.4	-4.4	-3.4
	reactions in nodes	-2.4	4.4	3.4
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 8	loads	0.4	-0.8	-1.8

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Calculation protocol				
	(kN)	X	Y	Z
	reactions in nodes	-0.4	0.8	1.8
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 9	loads	2.8	-5.2	-4.2
	reactions in nodes	-2.8	5.2	4.2
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 10	loads	-2.4	4.4	1.3
	reactions in nodes	2.4	-4.4	-1.3
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 11	loads	2.4	-4.4	-3.4
	reactions in nodes	-2.4	4.4	3.4
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 12	loads	0.0	0.0	-1.0
	reactions in nodes	-0.0	0.0	1.0
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 13	loads	0.0	0.0	-1.0
	reactions in nodes	0.0	0.0	1.0
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0

#### 4. Deflection

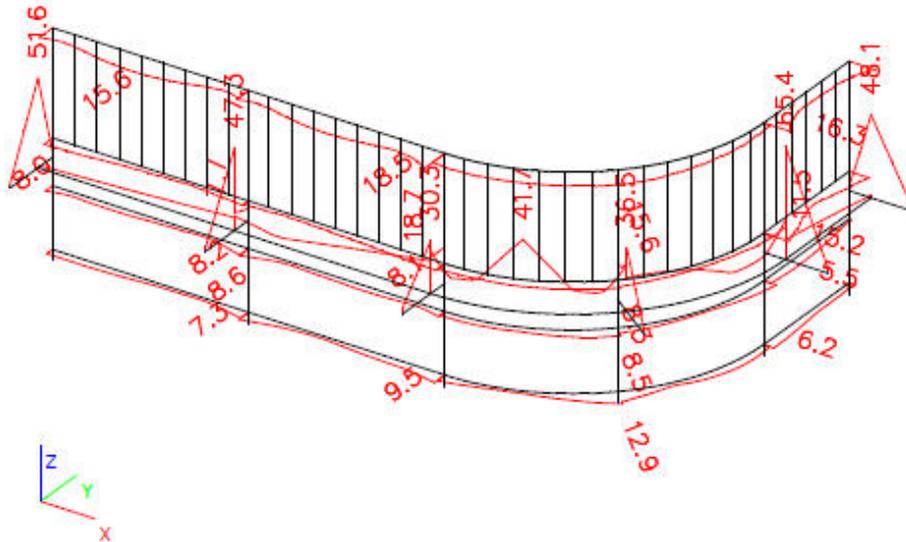
##### 4.1. Deformations on member; uz: NC101





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## 5.2. Stress; von Mises: Horizontal members



Nonlinear calculation, Extreme : Global

Selection : All

Nonlinear combinations : NC211

Cross-section : T1 - Tube (50; 3)

Values : Normal -, Normal +, Shear, von Mises, Fatigue, Kappa, Sigma Y

Member	Case	dx [m]	Normal - [MPa]	Normal + [MPa]	Shear [MPa]	von Mises [MPa]	Fatigue [MPa]	Kappa [-]
B135	NC211	0.588	-14.4		1.4	14.6		
B131	NC211	0.454	15.0	15.0			0.0	1.00
B133	NC211	0.899	-7.4	-7.4			0.0	1.00
B135	NC211	0.000		16.0	1.8	16.3		
B131	NC211	0.000	-3.5	-3.5			0.0	1.00
B133	NC211	0.899	-10.7		6.0	15.0		
B133	NC211	0.899		15.7	5.7	18.5		
B131	NC211	0.000	-3.5	-3.5	3.3	6.6		

Nonlinear calculation, Extreme : Global

Selection : All

Nonlinear combinations : NC211

Cross-section : T2 - O (50; 3; 50; 3)

Values : Normal -, Normal +, Shear, von Mises, Fatigue, Kappa, Sigma Y

Member	Case	dx [m]	Normal - [MPa]	Normal + [MPa]	Shear [MPa]	von Mises [MPa]	Fatigue [MPa]	Kappa [-]
B10	NC211	0.000	-15.2		0.0	15.2		
B10	NC211	0.000	15.2	15.2			0.0	1.00
B10	NC211	0.588	-11.4	-11.4			0.0	1.00
B10	NC211	0.000		15.2	0.0	15.2		
B8	NC211	0.000	2.3	2.3			0.0	1.00
B8	NC211	0.587		2.3	3.7	6.9		
B8	NC211	0.000	2.3	2.3	0.0	2.3		

Nonlinear calculation, Extreme : Global

Selection : All

Nonlinear combinations : NC211

Cross-section : T3 - O (70; 3; 30; 3)

Values : Normal -, Normal +, Shear, von Mises, Fatigue, Kappa, Sigma Y

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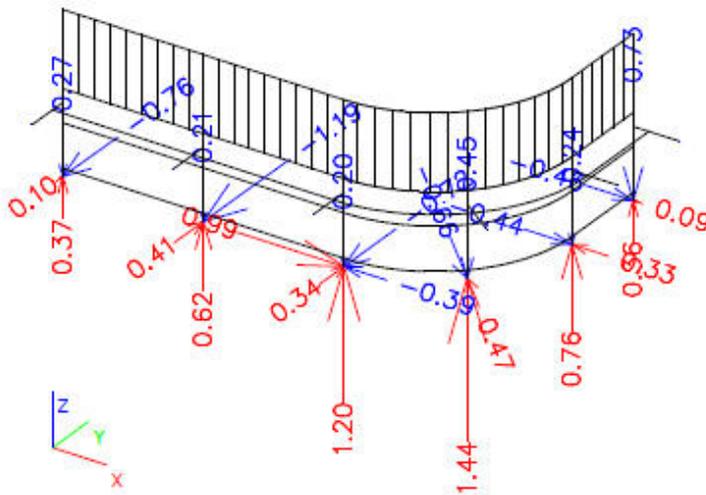
Member	Case	dx [m]	Normal - [MPa]	Normal + [MPa]	Shear [MPa]	von Mises [MPa]	Fatigue [MPa]	Kappa [-]
B9	NC211	0.383	-41.7		0.0	41.7		
B9	NC211	0.383	21.9	21.9			0.0	1.00
B9	NC211	0.868	-20.1	-20.1			0.0	1.00
B9	NC211	0.383			0.0	36.7		
B1	NC211	0.000	-1.6	-1.6			0.0	1.00
B9	NC211	0.150	-5.4		7.1	13.4		
B1	NC211	0.000	-1.6	-1.6	0.0	1.6		

Nonlinear calculation, Extreme : Global  
 Selection : All  
 Nonlinear combinations : NC211  
 Cross-section : T4 - Rectangle (50; 12)  
 Values : Normal -, Normal +, Shear, von Mises, Fatigue, Kappa, Sigma Y

Member	Case	dx [m]	Normal - [MPa]	Normal + [MPa]	Shear [MPa]	von Mises [MPa]	Fatigue [MPa]	Kappa [-]
B49	NC211	0.100	-65.4		0.0	65.4		
B49	NC211	0.100	52.3	52.3			0.0	1.00
B52	NC211	0.100	-51.2	-51.2			0.0	1.00
B49	NC211	0.100		60.2	0.0	60.2		
B48	NC211	0.000	-0.6	-0.6			0.0	1.00
B50	NC211	0.000	-1.7		12.0	20.8		
B48	NC211	0.000	-0.6	-0.6	0.0	0.6		

## 6. Factored reaction forces

### 6.1. Reactions @ Base Support: ULS Envelope



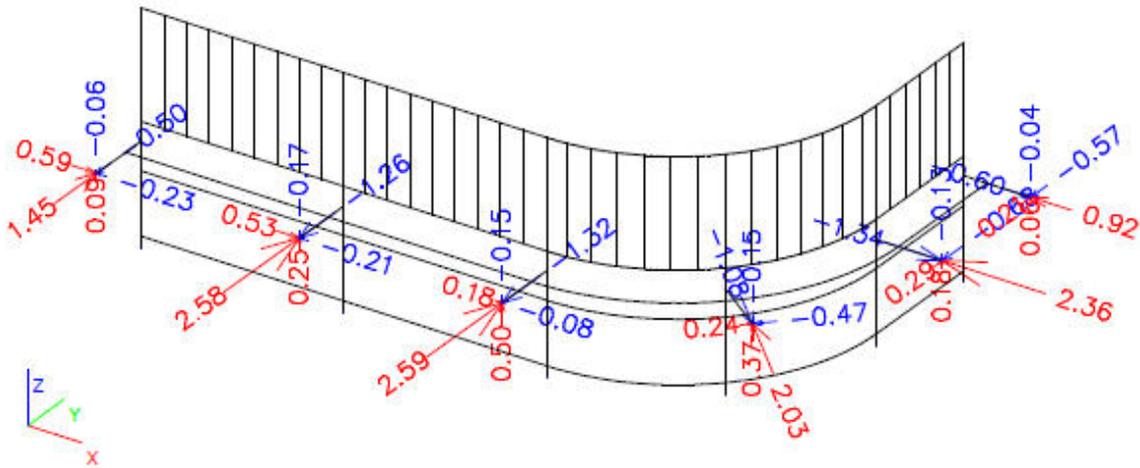
Nonlinear calculation, Extreme : Global  
 Selection : Named selection - Support - base  
 Class : Ultimate  
 Rotated supports

Support	Case	Rx [kN]	Ry [kN]	Rz [kN]	Mx [kNcm]	My [kNcm]	Mz [kNcm]
Sn9/N10	NC232	-0.39	0.01	0.15	0.00	0.00	0.00
Sn9/N10	NC231	0.99	-0.04	0.21	0.00	0.00	0.00
Sn5/N8	NC211	0.00	-1.19	0.03	0.00	0.00	0.00
Sn26/N255	NC224	0.00	0.47	0.58	0.00	0.00	0.00
Sn9/N12	NC222	0.00	-0.32	-0.73	0.00	0.00	0.00
Sn26/N255	NC222	0.00	0.17	1.44	0.00	0.00	0.00

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Support	Case	Rx [kN]	Ry [kN]	Rz [kN]	Mx [kNcm]	My [kNcm]	Mz [kNcm]
Sn26/N255	NC211	0.00	-0.53	0.80	0.00	0.00	0.00

### 6.2. Reactions @ Strut Support: ULS Envelope



Nonlinear calculation, Extreme : Global  
 Selection : Named selection - Support - strut  
 Class : Ultimate  
 Rotated supports

Support	Case	Rx [kN]	Ry [kN]	Rz [kN]	Mx [kNcm]	My [kNcm]	Mz [kNcm]
Sn19/N93	NC231	-0.68	2.28	0.02	0.00	0.00	0.00
Sn16/N99	NC231	0.59	0.06	0.02	0.00	0.00	0.00
Sn19/N93	NC223	0.16	-1.34	-0.11	0.00	0.00	0.00
Sn18/N95	NC212	0.10	2.59	0.49	0.00	0.00	0.00
Sn17/N97	NC213	0.33	-1.26	-0.17	0.00	0.00	0.00
Sn18/N95	NC222	-0.03	1.69	0.50	0.00	0.00	0.00
Sn16/N99	NC211	0.29	0.96	0.01	0.00	0.00	0.00

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## 2.5 Type C Balustrade

Refer to structural analysis results in section 2.5.3.

### 2.5.1 Deflection check

$$\begin{aligned}\delta_{max} &= 5.2 \text{ mm} \\ \delta_{allow} = \min\{1100/65; 15\} &= 15.0 \text{ mm} \quad \underline{0.35 < 1.0}\end{aligned}$$

### 2.5.2 Stress check to BS EN 1999-1-1

#### i Vertical members

$$\text{Max. Von mises stress, } \sigma_{max} = 72.6 \text{ N/mm}^2$$

#### EN AW-6082 T6

$$\sigma_{HAZ,Rd} = 0.5 \cdot 250 / 1.1 = 125.0 \text{ N/mm}^2 \quad \underline{0.58 < 1.0}$$

#### ii Horizontal members

$$\text{Max. Von mises stress, } \sigma_{max} = 50.6 \text{ N/mm}^2$$

$$\text{Max. HAZ stress, } \sigma_{HAZ} = 28.8 \text{ N/mm}^2$$

#### EN AW-6060 T6

$$\sigma_{el,Rd} = 140 / 1.1 = 127.27 \text{ N/mm}^2 \quad \underline{0.40 < 1.0}$$

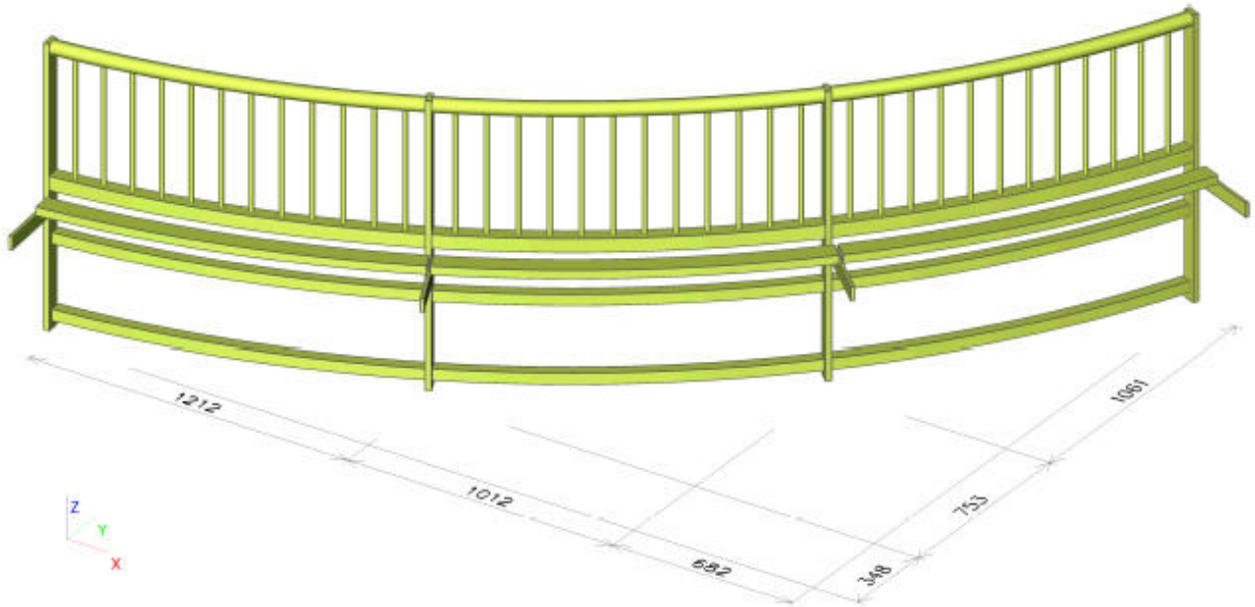
$$\sigma_{HAZ,Rd} = 0.43 \cdot 140 / 1.1 = 54.73 \text{ N/mm}^2 \quad \underline{0.53 < 1.0}$$

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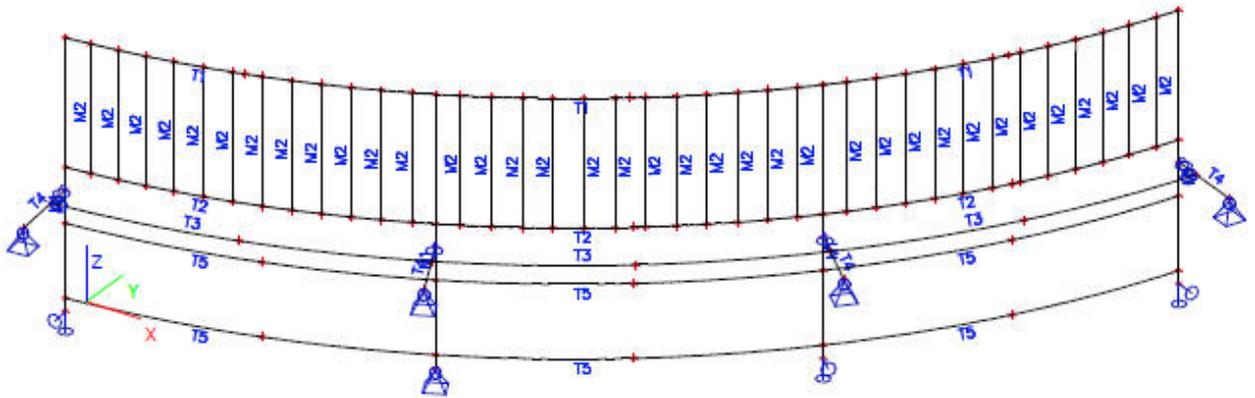
2.5.3 Structural analysis – Type C balustrade

	Project	The Corniche, London
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1. Structural model



1.1. Parameters

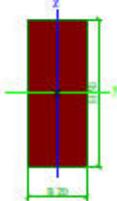


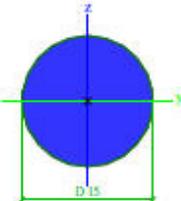
1.2. Materials

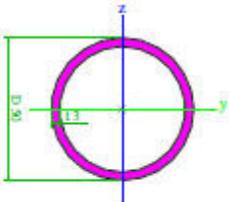
Name	Type	Unit mass [kg/m³]	E mod [MPa]	Poisson - nu	G mod [MPa]	Thermal exp [m/mK]
EN-AW 6060 T6	Aluminium	2700.00	7.0000e+04	0.3	2.6923e+04	0.00
EN-AW 6062 T6	Aluminium	2700.00	7.0000e+04	0.3	2.6923e+04	0.00

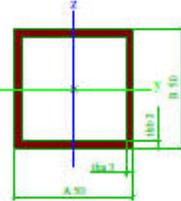
	Project	The Corniche, London
	Part	BAL-102 Type C
	Description	Aluminium - Welded assembly
	National code	EC - EN

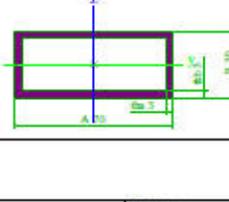
### 1.3. Cross-sections

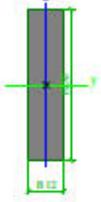
	Name	M1	A [cm <sup>2</sup> ]	10.00	
	Type	Rectangle	A y, z [cm <sup>2</sup> ]	8.33	8.33
	Detailed	50; 20	I y, z [cm <sup>4</sup> ]	20.83	3.33
	Item material	EN-AW 6082 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	3.59	9.95
	Fabrication	general	Wei y, z [cm <sup>2</sup> ]	8.33	3.33
	Use 2D FEM analysis	✓	Wpl y, z [cm <sup>2</sup> ]	12.50	5.00
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	10	25
		α [deg]	0.00		

	Name	M2	A [cm <sup>2</sup> ]	1.77	
	Type	Circle	A y, z [cm <sup>2</sup> ]	1.59	1.59
	Detailed	15	I y, z [cm <sup>4</sup> ]	0.25	0.25
	Item material	EN-AW 6082 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	0.00	0.50
	Fabrication	general	Wei y, z [cm <sup>2</sup> ]	0.33	0.33
	Use 2D FEM analysis	✓	Wpl y, z [cm <sup>2</sup> ]	0.56	0.56
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	7	8
		α [deg]	0.00		

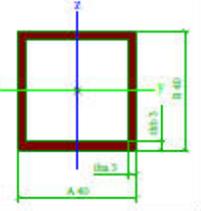
	Name	T1	A [cm <sup>2</sup> ]	4.43	
	Type	Tube	A y, z [cm <sup>2</sup> ]	2.97	2.97
	Detailed	50; 3	I y, z [cm <sup>4</sup> ]	12.28	12.28
	Item material	EN-AW 6060 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	0.00	23.96
	Fabrication	extrusion	Wei y, z [cm <sup>2</sup> ]	4.91	4.91
	Use 2D FEM analysis	✓	Wpl y, z [cm <sup>2</sup> ]	6.64	6.64
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	25	25
		α [deg]	0.00		

	Name	T2	A [cm <sup>2</sup> ]	5.64	
	Type	O	A y, z [cm <sup>2</sup> ]	2.90	2.90
	Detailed	50; 3; 50; 3	I y, z [cm <sup>4</sup> ]	20.85	20.85
	Item material	EN-AW 6060 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	0.09	30.69
	Fabrication	extrusion	Wei y, z [cm <sup>2</sup> ]	8.34	8.34
	Use 2D FEM analysis	✓	Wpl y, z [cm <sup>2</sup> ]	9.95	9.95
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	25	25
		α [deg]	0.00		

	Name	T3	A [cm <sup>2</sup> ]	5.64	
	Type	O	A y, z [cm <sup>2</sup> ]	3.92	1.78
	Detailed	70; 3; 30; 3	I y, z [cm <sup>4</sup> ]	8.38	33.32
	Item material	EN-AW 6060 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	7.98	20.67
	Fabrication	extrusion	Wei y, z [cm <sup>2</sup> ]	5.58	9.52
	Use 2D FEM analysis	✓	Wpl y, z [cm <sup>2</sup> ]	6.53	12.17
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	35	15
		α [deg]	0.00		

	Name	T4	A [cm <sup>2</sup> ]	6.00	
	Type	Rectangle	A y, z [cm <sup>2</sup> ]	5.00	5.00
	Detailed	50; 12	I y, z [cm <sup>4</sup> ]	12.50	0.72
	Item material	EN-AW 6060 T6	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	1.18	2.44
	Fabrication	general	Wei y, z [cm <sup>2</sup> ]	5.00	1.20
	Use 2D FEM analysis	✓	Wpl y, z [cm <sup>2</sup> ]	7.50	1.80
			d y, z [mm]	0	0
			c YUCS, ZUCS [mm]	6	25
		α [deg]	0.00		

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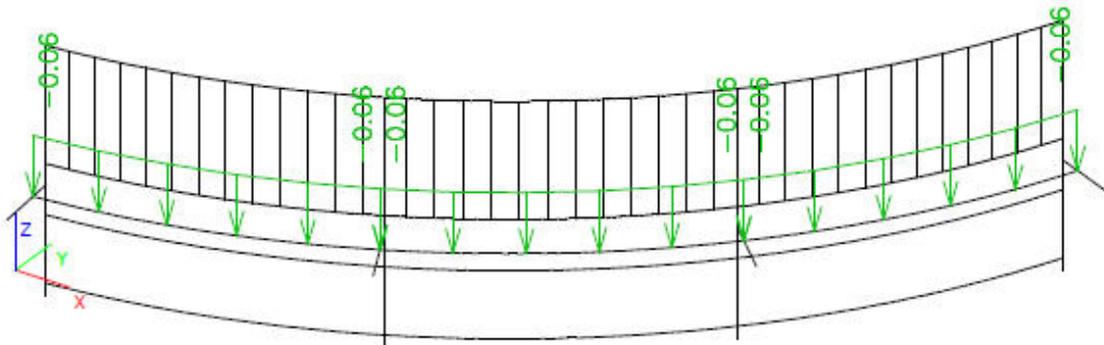
	Name	T5	A [cm²]	4.44
	Type	O	A y, z [cm²]	2.31 2.31
	Detailed	40; 3; 40; 3	I y, z [cm⁴]	10.20 10.20
	Item material	EN-AW 6060 T6	I w [cm⁴], i [cm⁴]	0.04 15.12
	Fabrication	extrusion	Wei y, z [cm²]	5.10 5.10
	Use 2D FEM analysis	✓	Wpl y, z [cm²]	6.17 6.17
			d y, z [mm]	0 0
			c YUCS, ZUCS [mm]	20 20
			α [deg]	0.00

Name	Mass [kg]	Surface [m²]	Volume [cm³]
Total results :	46.73	4.901	17306.41

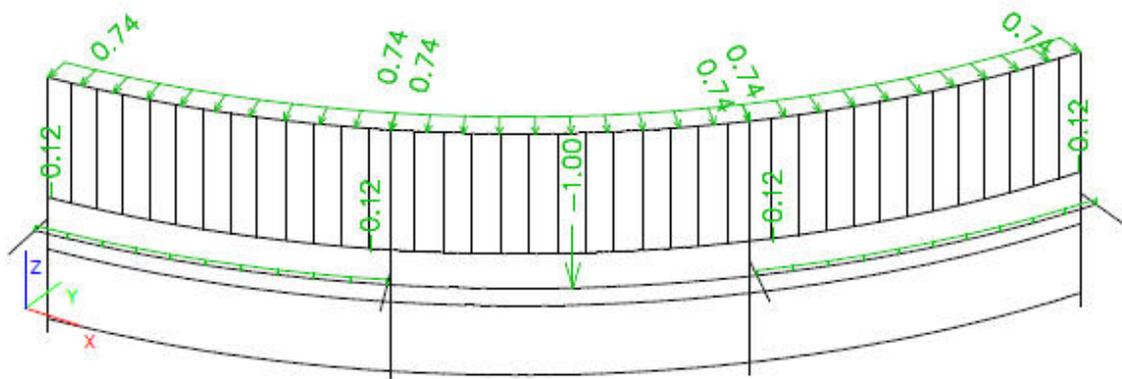
Material	Mass [kg]	Surface [m²]	Unit volume mass [kg/m³]	Volume [cm³]
EN-AW 6060 T6	27.33	3.504	2700.00	10121.60
EN-AW 6082 T6	19.40	1.397	2700.00	7184.81

## 2. Loads [kN, kN/m]

### 2.1. LC1 - D: Dead load



### 2.2. LC2 - I: Imposed load

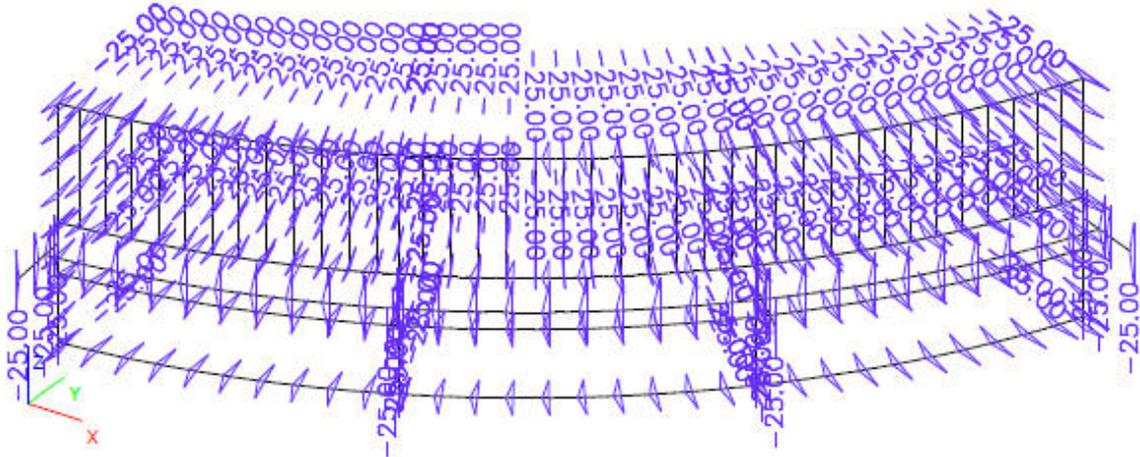




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## 2.5. LC5 - T2: Thermal load - Winter



## 2.6. Nonlinear combinations

Name	Description	Type	Load cases	Coeff. [ ]
NC101	(D) + (L)	Serviceability	LC0 - Selfweight	1.00
			LC1 - D - Dead load	1.00
			LC2 - L - Imposed/Live Load	1.00
NC102	(D) + (Wp)	Serviceability	LC0 - Selfweight	1.00
			LC1 - D - Dead load	1.00
			LC3 - W - Wind load Load	-1.00
NC103	(D) + (Ws)	Serviceability	LC0 - Selfweight	1.00
			LC1 - D - Dead load	1.00
			LC3 - W - Wind load Load	1.00
NC211	1.35(D) + 1.5(L) + 0.75(Wp) + 0.75(Ts)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC2 - L - Imposed/Live Load	1.50
			LC3 - W - Wind load Load	-0.75
			LC4 - T1 - Temperature Load, Summer	0.75
NC212	1.35(D) + 1.5(L) + 0.75(Ws) + 0.75(Ts)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC2 - L - Imposed/Live Load	1.50
			LC3 - W - Wind load Load	0.75
			LC4 - T1 - Temperature Load, Summer	0.75
NC213	1.35(D) + 1.5(Wp) + 0.9(Ts)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC3 - W - Wind load Load	-1.50
			LC4 - T1 - Temperature Load, Summer	0.90
NC214	1.35(D) + 1.5(Ws) + 0.9(Ts)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC3 - W - Wind load Load	1.50
			LC4 - T1 - Temperature Load, Summer	0.90
NC221	1.35(D) + 1.5(L) + 0.75(Wp) + 0.75(Tw)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC2 - L - Imposed/Live Load	1.50
			LC3 - W - Wind load Load	-0.75
			LC5 - T2 - Temperature Load, Winter	0.75
NC222	1.35(D) + 1.5(L) + 0.75(Ws) + 0.75(Tw)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC2 - L - Imposed/Live Load	1.50
			LC3 - W - Wind load Load	0.75

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Name	Description	Type	Load cases	Coef. [-]
NC222	1.35(D) + 1.5(L) + 0.75(Ws) + 0.75(Tw)	Ultimate	LC5 - T2 - Temperature Load, Winter	0.75
NC223	1.35(D) + 1.5(Wp) + 0.9(Tw)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC3 - W - Wind load Load	-1.50
			LC5 - T2 - Temperature Load, Winter	0.90
NC224	1.35(D) + 1.5(Ws) + 0.9(Tw)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC3 - W - Wind load Load	1.50
			LC5 - T2 - Temperature Load, Winter	0.90
NC231	1.35(D) + 1.5(Ts)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC4 - T1 - Temperature Load, Summer	1.50
NC232	1.35(D) + 1.5(Tw)	Ultimate	LC0 - Selfweight	1.35
			LC1 - D - Dead load	1.35
			LC5 - T2 - Temperature Load, Winter	1.50

## 2.7. Result classes

Name	List
Serviceability	NC101
	NC102
	NC103
Ultimate	NC211
	NC212
	NC213
	NC214
	NC221
	NC222
	NC223
	NC224
	NC231
	NC232

## 3. Calculation protocol

Calculation protocol				
<b>Nonlinear calculation</b>				
Number of 2D elements			0	
Number of 1D elements			896	
Number of mesh nodes			849	
Number of equations			5094	
Maximum iterations			50	
Number of increments			2	
Type of nonlinearity			II. order	
			local nonlinearities	
Method (II. order)			Newton-Raphson	
Bending theory			Mindlin	
<b>No. of combination</b>	<b>Start</b>	<b>End</b>	<b>No. of iterations</b>	
NC 1	26.05.2015 13:34	26.05.2015 13:34	4	
NC 2	26.05.2015 13:34	26.05.2015 13:34	6	
NC 3	26.05.2015 13:34	26.05.2015 13:34	4	
NC 4	26.05.2015 13:34	26.05.2015 13:34	4	
NC 5	26.05.2015 13:34	26.05.2015 13:34	6	
NC 6	26.05.2015 13:34	26.05.2015 13:35	5	
NC 7	26.05.2015 13:35	26.05.2015 13:35	5	
NC 8	26.05.2015 13:35	26.05.2015 13:35	5	
NC 9	26.05.2015 13:35	26.05.2015 13:35	5	
NC 10	26.05.2015 13:35	26.05.2015 13:35	5	
NC 11	26.05.2015 13:35	26.05.2015 13:35	5	
NC 12	26.05.2015 13:35	26.05.2015 13:35	5	
NC 13	26.05.2015 13:35	26.05.2015 13:35	5	
<b>Sum of loads and reactions.</b>				
	<b>[kN]</b>	<b>X</b>	<b>Y</b>	<b>Z</b>
Nonlinear combination 1	loads	1.6	-2.1	-2.0
	reactions in nodes	-1.6	2.1	2.0

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Calculation protocol				
	[kN]	X	Y	Z
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 2	loads	0.6	-0.8	-1.8
	reactions in nodes	-0.6	0.8	1.8
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 3	loads	-2.4	3.2	0.9
	reactions in nodes	2.4	-3.2	-0.9
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 4	loads	2.4	-3.2	-2.3
	reactions in nodes	-2.4	3.2	2.3
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 5	loads	4.2	-5.6	-4.2
	reactions in nodes	-4.2	5.6	4.2
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 6	loads	-3.5	4.8	1.4
	reactions in nodes	3.5	-4.8	-1.4
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 7	loads	3.5	-4.8	-3.3
	reactions in nodes	-3.5	4.8	3.3
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 8	loads	0.6	-0.8	-1.8
	reactions in nodes	-0.6	0.8	1.8
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 9	loads	4.2	-5.6	-4.2
	reactions in nodes	-4.2	5.6	4.2
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 10	loads	-3.5	4.8	1.4
	reactions in nodes	3.5	-4.8	-1.4
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 11	loads	3.5	-4.7	-3.3
	reactions in nodes	-3.5	4.7	3.3
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 12	loads	0.0	0.0	-0.9
	reactions in nodes	-0.0	-0.0	0.9
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0
Nonlinear combination 13	loads	0.0	0.0	-0.9
	reactions in nodes	0.0	0.0	0.9
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	0.0	0.0	0.0





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Member	Case	dx [m]	Normal - [MPa]	Normal + [MPa]	Shear [MPa]	von Mises [MPa]	Fatigue [MPa]	Kappa [-]
B125	NC211	0.000	-17.4		0.0	17.4		
B125	NC211	0.000	16.6	16.6			0.0	1.00
B125	NC211	1.268	-14.2	-14.2			0.0	1.00
B125	NC211	0.000		17.1	0.0	17.1		
B122	NC211	0.000	-13.1	-13.1			0.0	1.00
B125	NC211	1.268	-0.1		5.6	9.7		
B122	NC211	0.000	-13.1	-13.1	0.0	13.1		

Nonlinear calculation, Extreme : Global

Selection : All

Nonlinear combinations : NC211

Cross-section : T3 - O (70; 3; 30; 3)

Values : Normal -, Normal +, Shear, von Mises, Fatigue, Kappa, Sigma Y

Member	Case	dx [m]	Normal - [MPa]	Normal + [MPa]	Shear [MPa]	von Mises [MPa]	Fatigue [MPa]	Kappa [-]
B126	NC211	0.852	-57.4		0.0	57.4		
B126	NC211	0.852	50.9	50.9			0.0	1.00
B126	NC211	0.000	-27.3	-27.3			0.0	1.00
B126	NC211	0.852		54.1	0.0	54.1		
B123	NC211	0.000	-1.2	-1.2			0.0	1.00
B126	NC211	1.014		0.1	6.2	10.8		
B123	NC211	0.000	-1.2	-1.2	0.0	1.2		

Nonlinear calculation, Extreme : Global

Selection : All

Nonlinear combinations : NC211

Cross-section : T4 - Rectangle (50; 12)

Values : Normal -, Normal +, Shear, von Mises, Fatigue, Kappa, Sigma Y

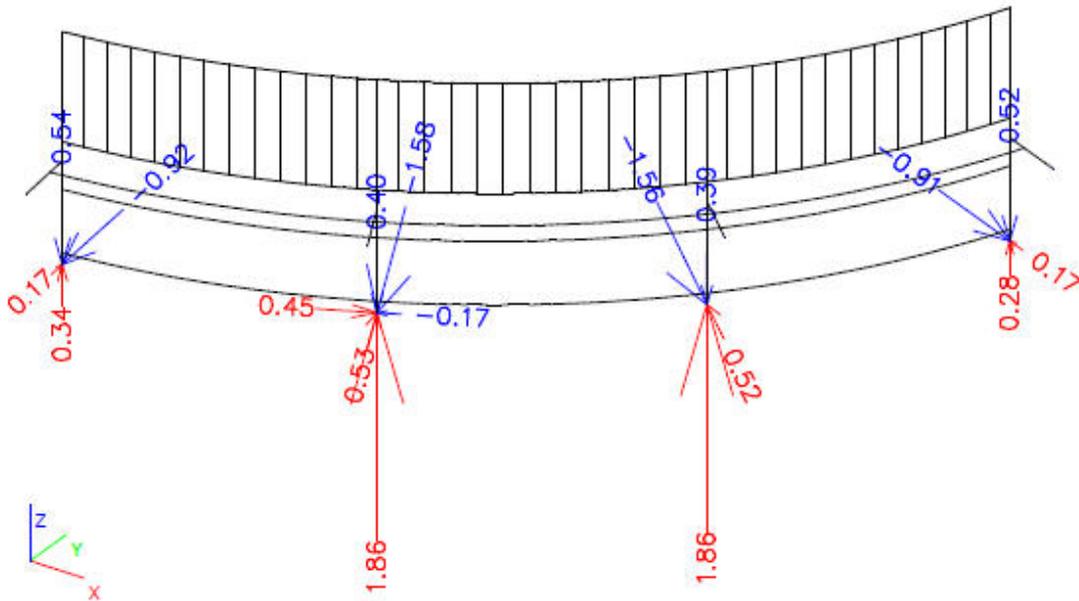
Member	Case	dx [m]	Normal - [MPa]	Normal + [MPa]	Shear [MPa]	von Mises [MPa]	Fatigue [MPa]	Kappa [-]
B95	NC211	0.100	-50.6		0.0	50.6		
B95	NC211	0.100	43.6	43.6			0.0	1.00
B58	NC211	0.100	-42.8	-42.8			0.0	1.00
B94	NC211	0.100		46.1	0.0	46.1		
B58	NC211	0.000	-1.7	-1.7			0.0	1.00
B98	NC211	0.100	-18.8		23.9	45.4		
B58	NC211	0.000	-1.7	-1.7	0.0	1.7		

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## 6. Factored reaction forces

### 6.1. Reactions @ Base Support: ULS Envelope



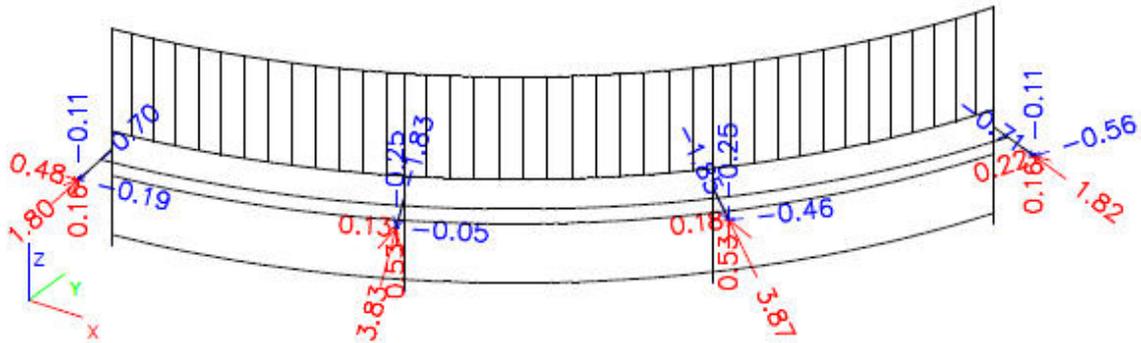
Nonlinear calculation, Extreme : Global  
 Selection : Named selection - Support - base  
 Class : Ultimate  
 Rotated supports

Support	Case	Rx [kN]	Ry [kN]	Rz [kN]	Mx [kNcm]	My [kNcm]	Mz [kNcm]
Sn10/N104	NC232	-0.17	0.01	0.30	0.00	0.00	0.00
Sn10/N104	NC231	0.45	-0.06	0.18	0.00	0.00	0.00
Sn10/N104	NC211	0.23	-1.58	1.17	0.00	0.00	0.00
Sn10/N104	NC224	-0.10	0.53	0.90	0.00	0.00	0.00
Sn9/N100	NC221	0.00	-0.89	-0.54	0.00	0.00	0.00
Sn10/N104	NC222	-0.07	-1.01	1.86	0.00	0.00	0.00
Sn12/N108	NC211	0.00	-0.91	-0.50	0.00	0.00	0.00

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### 6.2. Reactions @ Strut Support: ULS Envelope



Nonlinear calculation, Extreme : Global  
 Selection : Named selection - Support - strut  
 Class : Ultimate  
 Rotated supports

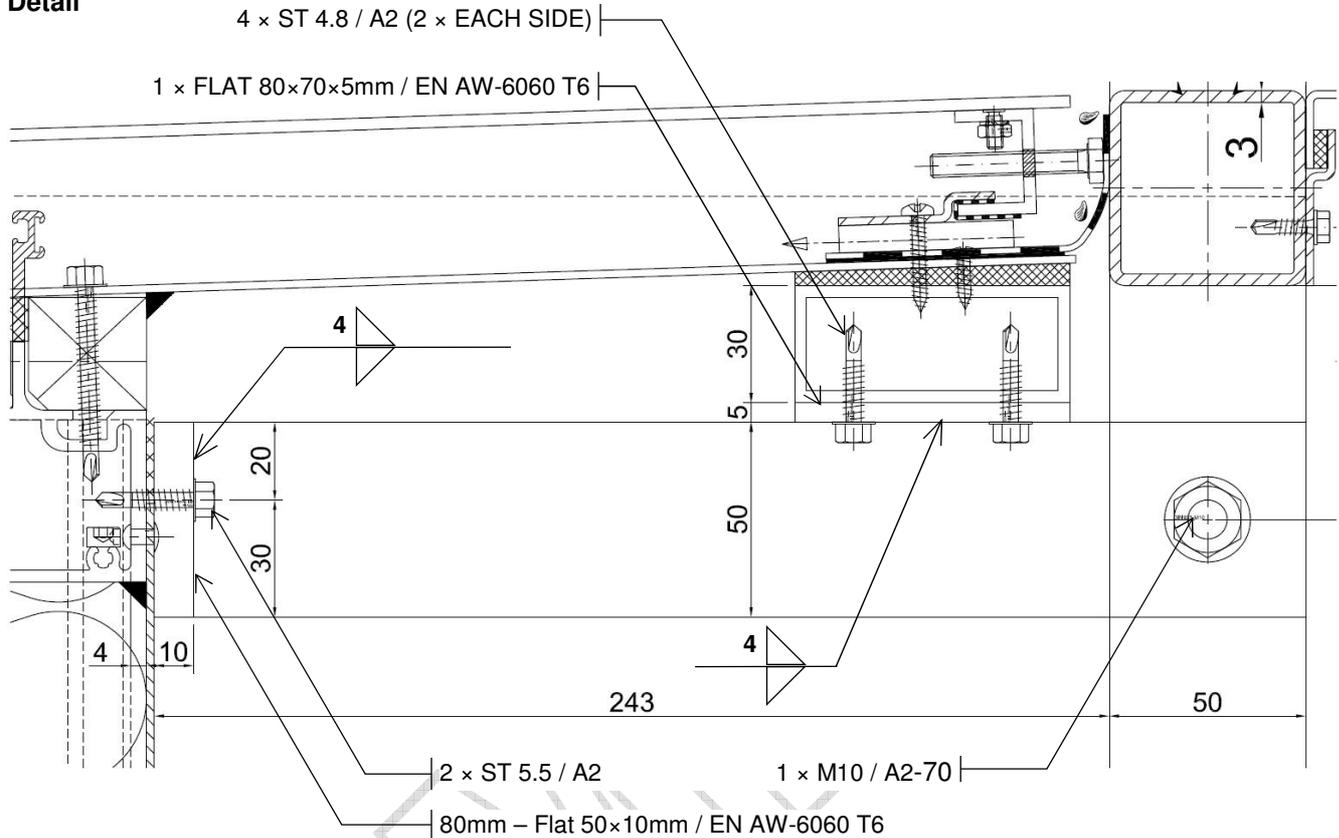
Support	Case	Rx [kN]	Ry [kN]	Rz [kN]	Mx [kNm]	My [kNm]	Mz [kNm]
Sn24/N183	NC231	-0.56	0.16	0.02	0.00	0.00	0.00
Sn21/N111	NC231	0.48	0.14	0.02	0.00	0.00	0.00
Sn23/N185	NC223	0.11	-1.85	-0.24	0.00	0.00	0.00
Sn23/N185	NC212	-0.21	3.87	0.53	0.00	0.00	0.00
Sn23/N185	NC213	-0.29	-1.57	-0.25	0.00	0.00	0.00
Sn22/N187	NC212	0.05	3.83	0.53	0.00	0.00	0.00
Sn24/N183	NC211	-0.27	1.14	-0.05	0.00	0.00	0.00

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### 3 Brackets and anchors

#### 3.1 Strut connection

##### Detail



##### 3.1.1 Design Forces

Refer to results of balustrade structural analysis in sections 2.3.3, 2.4.3 and 2.5.3,

$$\begin{aligned} \text{Max. } R_{x,Ed} &= 0.59 \text{ kN} \\ \text{Max. } R_{z,Ed} &= 0.58 \text{ kN} \\ \text{Max. } +R_{y,Ed} &= 3.87 \text{ kN} \\ \text{Max. } -R_{y,Ed} &= -1.85 \text{ kN} \end{aligned}$$

##### 3.1.2 Fixing to spandrel element

- i Self drilling screw check to BS EN 1999-1-4

$$\begin{aligned} F_{v,Ed} &= \sqrt{(0.59^2 + 0.58^2)} = 0.83 \text{ kN} \\ F_{t,Ed} &= 1.85/2 = 0.92 \text{ kN} \end{aligned}$$

##### 2 x ST 5.5 / A2

$$\begin{aligned} F_{v,Rd} &= 12.50 \cdot 380 / 1.25 = 3.80 \text{ kN} && \underline{0.22 < 1.0} \\ F_{t,Rd} &= 12.50 \cdot 560 / 1.25 = 5.60 \text{ kN} && \underline{0.16 < 1.0} \\ 0.22 + 0.16 / 1.4 &= && \underline{0.33 < 1.0} \\ F_{b,Rd} &= 1.5 \cdot 5.5 \cdot 4.0 \cdot 170 / 1.25 = 4.49 \text{ kN} && \underline{0.18 < 1.0} \end{aligned}$$

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$$F_{o,Rd} = 0.65 \cdot 5.5 \cdot 4.0 \cdot 170 / 1.25 = 1.94 \text{ kN} \quad \underline{0.47 < 1.0}$$

$$0.18 + 0.47 = \underline{0.65 < 1.0}$$

ii End plate check to BS EN 1999-1-1

$$V_{Ed} = 0.92$$

$$M_{Ed} = 0.92 \cdot 20 = 18.4 \text{ kN}\cdot\text{mm}$$

**80mm – Flat 50×10mm / EN AW-6060 T6**

$$M_{el,Rd} = 50 \cdot 10^2 / 6 \cdot 140 / 1.1 = 106.06 \text{ kN}\cdot\text{mm}$$

iii Weld check to BS EN 1999-1-1

$$\sigma_{w,Ed} = (3870 + 590) / (2 \cdot 2 \cdot 4 \cdot 50) = 5.58 \text{ N/mm}^2$$

$$\tau_{w\perp,Ed} = (3870 + 590) / (2 \cdot 2 \cdot 4 \cdot 50) = 5.58 \text{ N/mm}^2$$

$$\tau_{w//,Ed} = 580 / (2 \cdot 0.707 \cdot 4 \cdot 50) = 2.05 \text{ N/mm}^2$$

$$f_{w,Ed} = \sqrt{[5.58^2 + 3(5.58^2 + 2.05^2)]} = 11.71 \text{ N/mm}^2$$

**4mm fillet Back-to-back**

$$f_{w,Rd} = 0.59 \cdot 170 / 1.35 = 74.30 \text{ N/mm}^2 \quad \underline{0.16 < 1.00}$$

3.1.3 Fixing to balustrade

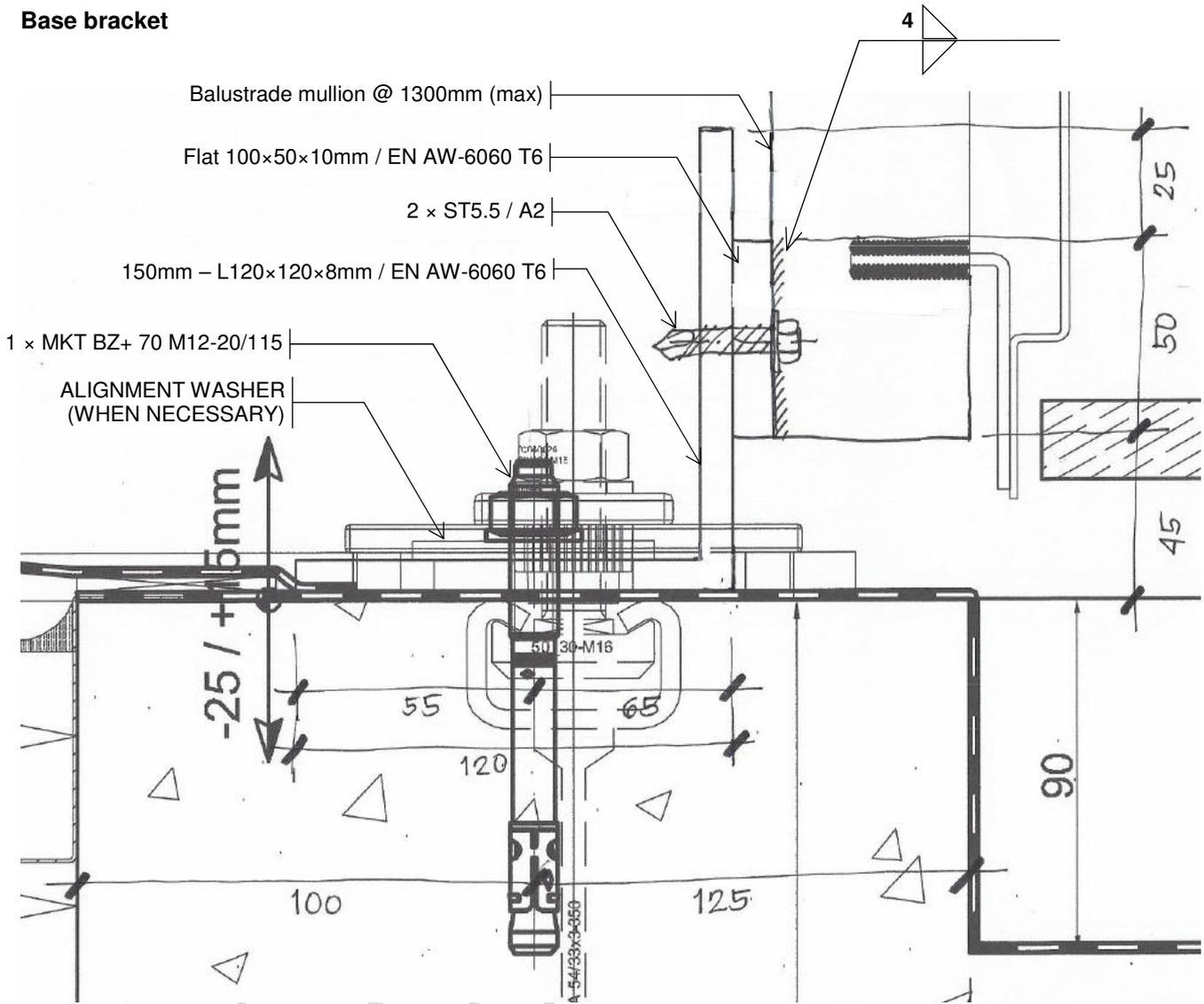
$$F_{v,Ed} = \sqrt{(3.87^2 + 0.58^2)} = 3.91 \text{ kN}$$

**1 × M10 / A2-70**

$$F_{v,Rd} = 0.5 \cdot 57.99 \cdot 700 / 1.25 = 16.24 \text{ kN} \quad \underline{0.24 < 1.0}$$

$$F_{b,Rd} = 1.5 \cdot 10 \cdot 10 \cdot 170 / 1.25 = 20.40 \text{ kN} \quad \underline{0.19 < 1.0}$$

### 3.2 Base bracket



#### 3.2.1 Design forces

$Max. R_{x,Ed} = 0.99 \text{ kN}$   
 $Max. R_{y,Ed} = 1.58 \text{ kN}$   
 $Max. R_{z,Ed} = 1.86 \text{ kN}$

#### 3.2.2 Fixing to balustrade

i Self-drilling screws check to BS EN 1999-1-4

$F_{v,Ed} = [\sqrt{(0.99^2 + 1.86^2)}] / 2 = 1.05 \text{ kN}$   
 $F_{t,Ed} = 1.58 / 2 + 0.99 \cdot 80 / 60 = 2.11 \text{ kN}$

#### 2 x ST5.5 / A2

$F_{v,Rd} = 12.50 \cdot 380 / 1.25 = 3.80 \text{ kN} \quad 0.28 < 1.0$   
 $F_{b,Rd} = 1.5 \cdot 5.5 \cdot 8 \cdot 170 / 1.25 = 8.98 \text{ kN} \quad 0.12 < 1.0$   
 $F_{t,Rd} = 12.50 \cdot 560 / 1.25 = 5.60 \text{ kN} \quad 0.38 < 1.0$   
 $F_{o,Rd} = 0.65 \cdot 5.5 \cdot 8 \cdot 170 / 1.25 = 3.89 \text{ kN} \quad 0.54 < 1.0$

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$$0.38 + 0.54 = \underline{0.92 < 1.0}$$

### 3.2.3 Fixing to slab

#### i Angle base plate

$$M_{Ed} = 1.58 \cdot 120 = 189.6 \text{ kN}\cdot\text{mm}$$

**150mm – L120×120×8mm / EN AW-6060 T6 or EN AW-5754 H24/34**

$$M_{el,Rd} = 150 \cdot 8^2 / 6 \cdot 140 / 1.1 = 203.64 \text{ kN}\cdot\text{mm} \quad \underline{0.93 < 1.0}$$

#### ii Anchors

$$V_{y,Sd} = 1.58 \text{ kN}$$

$$V_{x,Sd} = 0.99 \text{ kN}$$

$$M_{x,Sd} = 1.58 \cdot 0.12 = 0.19 \text{ kN}\cdot\text{m}$$

$$M_{y,Sd} = 0.99 \cdot 12 = 0.12 \text{ kN}\cdot\text{m}$$

**Provide: 1 × MKT BZ+ M12-20/115**

SAMPLE

Design office: Lindner Fassaden GmbH  
 Person in charge: LMC  
 Construction project: The Corniche, London  
 Number:  
 Position: BAL-102 Base anchor

**MKT**<sup>®</sup>  
 ... a solid connection  
 Date: 26/05/2015

**Input data:**

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**Concrete:**

cracked concrete  
 concrete grade: C20/25

**Reinforcement:**

normal or no reinforcement  
 without edge reinforcement  
 with limitation of crack width  $w_k \leq 0.3\text{mm}$

**Anchor bending:**

without anchor bending

**Installation conditions:**

hammer-drilled hole

**Static loads****Tensile load:**

$$N_{sd} = 0.00 \text{ kN}$$

**Shear load:**

$$V_{xsd} = 0.99 \text{ kN}$$

$$V_{ysd} = -1.58 \text{ kN}$$

**Moments:**

$$M_{xsd} = 0.19 \text{ kNm}$$

$$M_{ysd} = 0.12 \text{ kNm}$$

$$M_{zsd} = 0.00 \text{ kNm}$$

**Eccentric load**

$$e_x = 0.0 \text{ mm}$$

$$e_y = 0.0 \text{ mm}$$

**Anchor plate:**

$$x = 150 \text{ mm}$$

$$y = 80 \text{ mm}$$

$$l_{x1} = 75 \text{ mm}$$

$$l_{x2} = 75 \text{ mm}$$

$$l_{y1} = 40 \text{ mm}$$

$$l_{y2} = 40 \text{ mm}$$

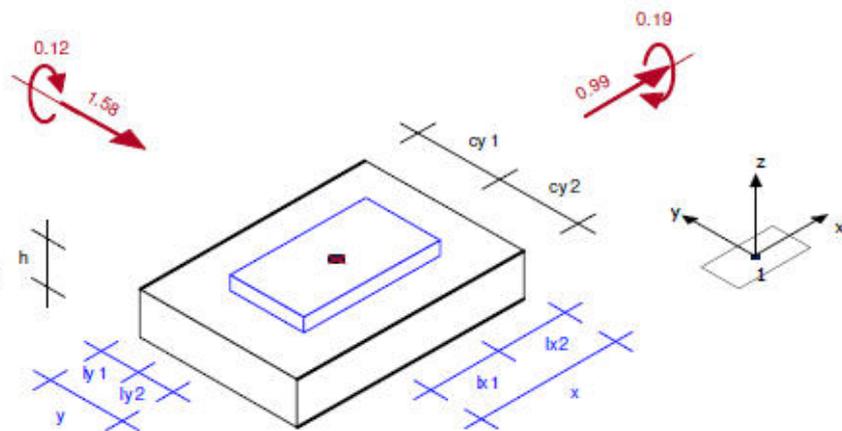
**Edge distances:**

$$q_{y1} = 100 \text{ mm}$$

$$q_{y2} = 85 \text{ mm}$$

**Thickness of base material:**

$$h = 315 \text{ mm}$$



[ kN, kNm ]

**Wedge Anchor BZ plus M12 hef 50**

Designed acc. to ETAG 001, Annex C

Official approval ETA-99/0010: MKT BZ plus / BZ-IG

Go ahead, design okay!

Anchor Design Program Version 4.21

MKT Metall-Kunststoff-Technik GmbH &amp; Co.KG - Auf dem Immei 2 - D-67685 Weierbach - Tel.: +49 (0) 63 74 / 91 16 -0 - Fax: +49 (0) 63 74 / 91 16 60

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## 4 Balustrades @ High Upstand

### 4.1 Members check

Refer to structural analysis in section 4.1.3.

#### 4.1.1 Deflection check

$$\delta_{max} = 14.8 \text{ mm}$$

$$\delta_{allow} = \min\{1100/65; 15\} = 15.0 \text{ mm} \quad \underline{0.91 < 1.0}$$

#### 4.1.2 Stress check to BS EN 1999-1-1

Maximum calculated Von Mises (or equivalent) stress in the analysis results.

##### i Members

$$\sigma_{max} = \sqrt{(\sigma^2_{normal} + 3\tau^2_{shear})} = 121.3 \text{ N/mm}^2$$

##### 15mm / EN AW-6060 T6

$$\sigma_{pl,Rd} = \min\{1.2 \cdot 140/1.1; 170/1.25\} = 136.0 \text{ N/mm}^2 \quad \underline{0.89 < 1.0}$$

##### ii Welded plates

$$Max. \sigma = \sqrt{(\sigma^2_{normal} + 3\tau^2_{shear})} = 119.9 \text{ N/mm}^2$$

$$Max. \sigma_{HAZ} = \sqrt{(\sigma^2_{normal} + 3\tau^2_{shear})} = 108.4 \text{ N/mm}^2$$

##### 15mm / EN AW-6082 T6

$$\sigma_{el,Rd} = 260 \text{ N/mm}^2 \quad \underline{0.52 < 1.0}$$

$$\sigma_{HAZ,Rd} = 0.48 \cdot 260/1.1 = 113.45 \text{ N/mm}^2 \quad \underline{0.96 < 1.0}$$

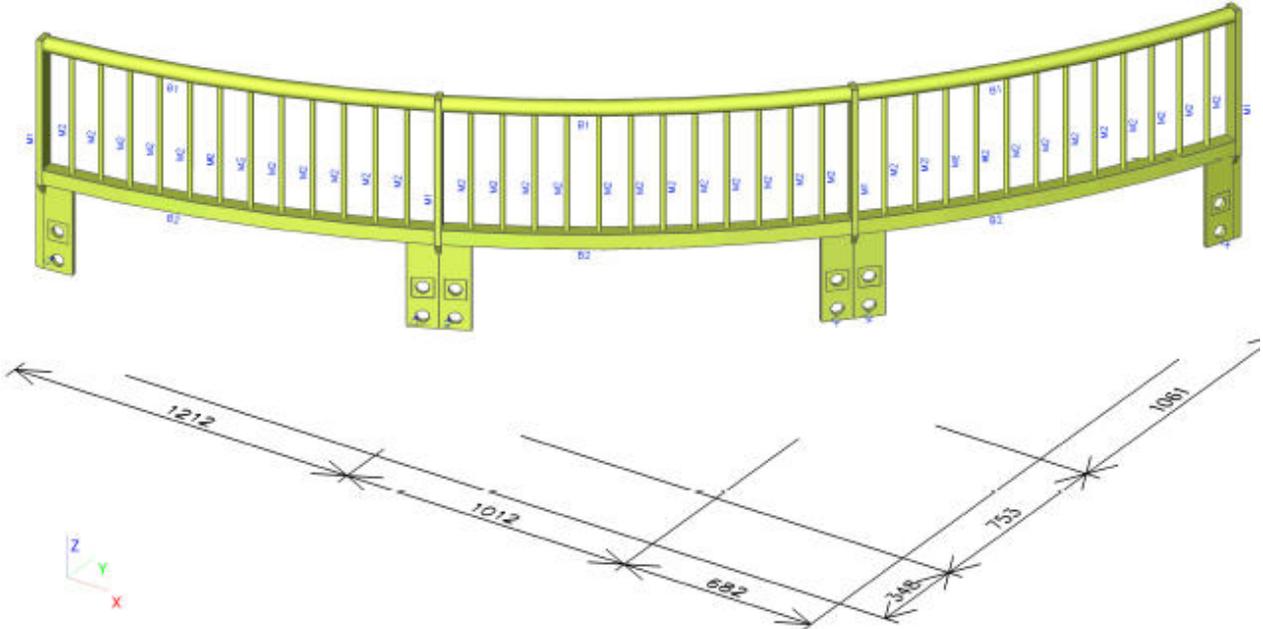
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### 4.1.3 Balustrade Type C – Structural analysis

	Project	The Corniche, London
	Part	BAL-102: Type C
	Description	Aluminium bolted assembly
	National code	EC - EN

## 1. Parameters

### 1.1. Structural model (rotated view)



### 1.2. Materials

Name	Unit mass [kg/m³]	E mod [MPa]	Poisson - nu	G mod [MPa]	Thermal exp [m/mK]
EN AW-6060 T6	2700.00	7.0000e+04	0.3	2.6923e+04	0.00

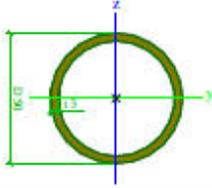
Name	Type	Unit mass [kg/m³]	E mod [MPa]	Poisson - nu	G mod [MPa]	Thermal exp [m/mK]
EN-AW 6060 T6	Aluminium	2700.00	7.0000e+04	0.3	2.6923e+04	0.00

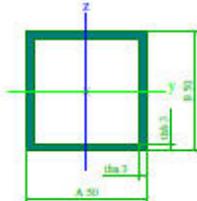
### 1.3. Cross-sections

	Name, Type	M1	Rectangle
	Detailed	50; 20	
	Item material, Fabrication	EN-AW 6062 T6	extrusion
	A [cm²]	10.00	
	A y, z [cm²]	8.33	8.33
	I y, z [cm⁴]	20.83	3.33
	I w [cm⁴], t [cm⁴]	3.59	9.95
	c YUCS, ZUCS [mm]	10	25
	Wei y, z [cm³]	8.33	3.33
	Wpl y, z [cm³]	12.50	5.00

	Name, Type	M2	Circle
	Detailed	15	
	Item material, Fabrication	EN-AW 6062 T6	extrusion
	A [cm²]	1.77	
	A y, z [cm²]	1.59	1.59
	I y, z [cm⁴]	0.25	0.25
	I w [cm⁴], t [cm⁴]	0.00	0.50
	c YUCS, ZUCS [mm]	7	8
	Wei y, z [cm³]	0.33	0.33
	Wpl y, z [cm³]	0.56	0.56

	Project	The Corniche, London
	Part	BAL-102: Type C
	Description	Aluminium bolted assembly
	National code	EC - EN

	Name, Type	B1	Tube
	Detailed	50; 3	
	Item material, Fabrication	EN-AW 6060 T6	extrusion
	A [cm <sup>2</sup> ]	4.43	
	A y, z [cm <sup>2</sup> ]	2.97	2.97
	I y, z [cm <sup>4</sup> ]	12.28	12.28
	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	0.00	23.98
	c YUCS, ZUCS [mm]	25	25
	Wei y, z [cm <sup>2</sup> ]	4.91	4.91
	Wpl y, z [cm <sup>2</sup> ]	6.64	6.64

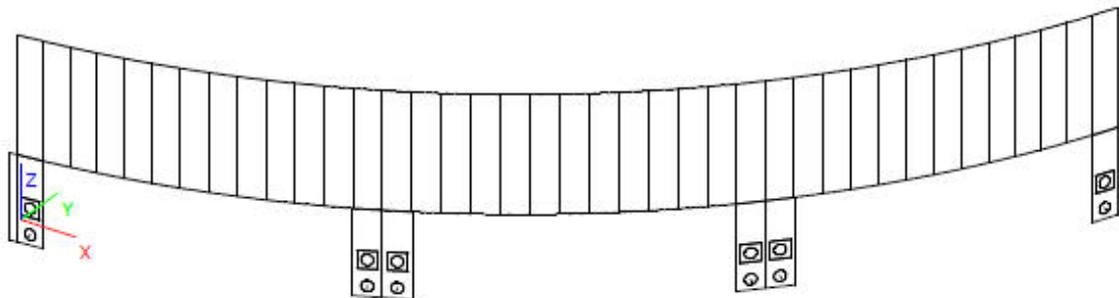
	Name, Type	B2	O
	Detailed	50; 3; 50; 3	
	Item material, Fabrication	EN-AW 6060 T6	extrusion
	A [cm <sup>2</sup> ]	5.64	
	A y, z [cm <sup>2</sup> ]	2.90	2.90
	I y, z [cm <sup>4</sup> ]	20.85	20.85
	I w [cm <sup>4</sup> ], t [cm <sup>4</sup> ]	0.09	30.69
	c YUCS, ZUCS [mm]	25	25
	Wei y, z [cm <sup>2</sup> ]	8.34	8.34
	Wpl y, z [cm <sup>2</sup> ]	9.95	9.95

Name	Mass [kg]	Surface [m <sup>2</sup> ]	Volume [cm <sup>3</sup> ]
Total results :	31.84	2.628	11793.31

Material	Mass [kg]	Surface [m <sup>2</sup> ]	Unit volume mass [kg/m <sup>3</sup> ]	Volume [cm <sup>3</sup> ]
EN-AW 6060 T6	2.40	0.062	2700.00	888.77
EN-AW 6060 T6	10.34	1.358	2700.00	3829.76
EN-AW 6082 T6	19.10	1.208	2700.00	7074.79

## 2. Loads [kN, kN/m]

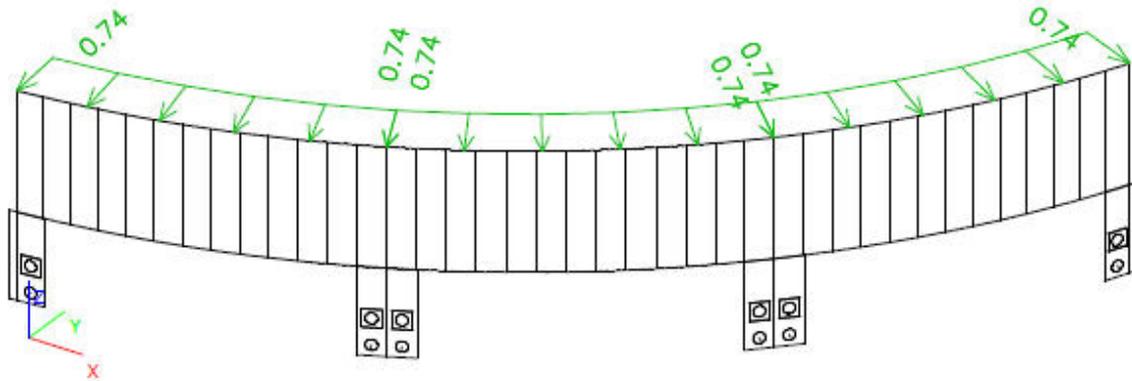
### 2.1. LC1: Dead load



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	Project	The Corniche, London
	Part	BAL-102: Type C
	Description	Aluminium bolted assembly
	National code	EC - EN

### 2.2. LC2: Imposed load



### 2.3. Nonlinear combinations

Name	Type	Load cases	Coeff. [-]
NC101	Serviceability	LC0 - Selfweight	1.00
		LC1 - D - Dead load	1.00
		LC2 - L - Imposed/Live Load	1.00
NC201	Ultimate	LC0 - Selfweight	1.35
		LC1 - D - Dead load	1.35
		LC2 - L - Imposed/Live Load	1.50

### 3. Calculation protocol

Calculation protocol
----------------------

#### Nonlinear calculation

Number of 2D elements	687
Number of 1D elements	870
Number of mesh nodes	1461
Number of equations	8766
Maximum iterations	50
Number of increments	1
Type of nonlinearity	II. order
Method (II. order)	local nonlinearities
Method (II. order)	Newton-Raphson
Bending theory	Mindlin

No. of combination	Start	End	No. of iterations
NC 1	18.05.2015 13:30	18.05.2015 13:33	11
NC 2	18.05.2015 13:33	18.05.2015 13:35	13

#### Sum of loads and reactions.

	[kN]	X	Y	Z
Nonlinear combination 1	loads	1.6	-2.1	-0.4
	reactions in nodes	4.9	-6.6	0.3
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	-6.5	8.7	0.1
Nonlinear combination 2	loads	2.4	-3.2	-0.6
	reactions in nodes	7.2	-9.9	0.4
	reactions on lines	0.0	0.0	0.0
	contact 1D	0.0	0.0	0.0
	contact 2D	-9.6	13.1	0.2

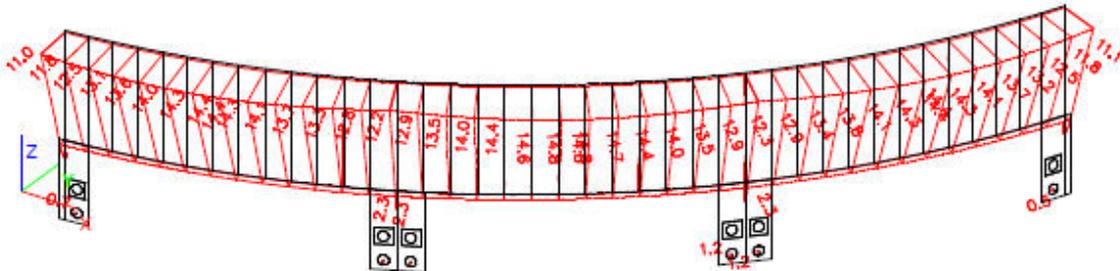
16.05.2015	3/5
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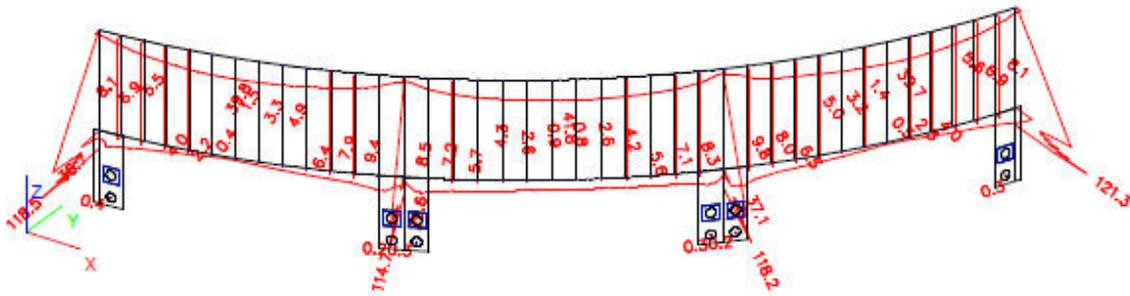
	Project	The Corniche, London
	Part	BAL-102: Type C
	Description	Aluminium bolted assembly
	National code	EC - EN

#### 4. Analysis results

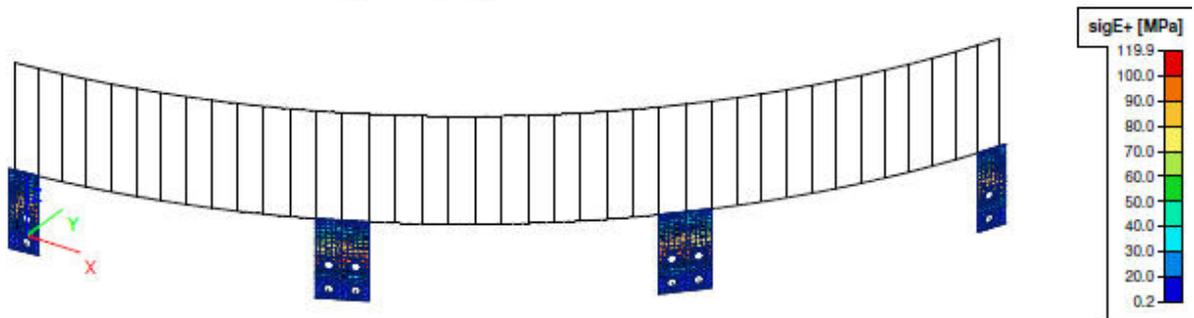
##### 4.1. Deformations on member, NC101; uz



##### 4.2. Stress, NC101; von Mises [N/mm<sup>2</sup>]



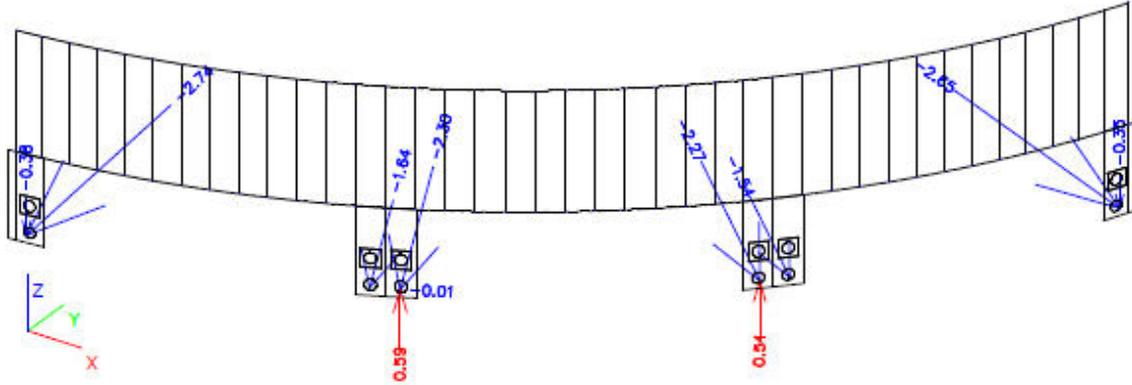
##### 4.3. 2D member - Factored Stresses, NC201; sigE+



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	Project	The Corniche, London
	Part	BAL-102: Type C
	Description	Aluminium bolted assembly
	National code	EC - EN

#### 4.4. Factored Reaction Forces [kN]

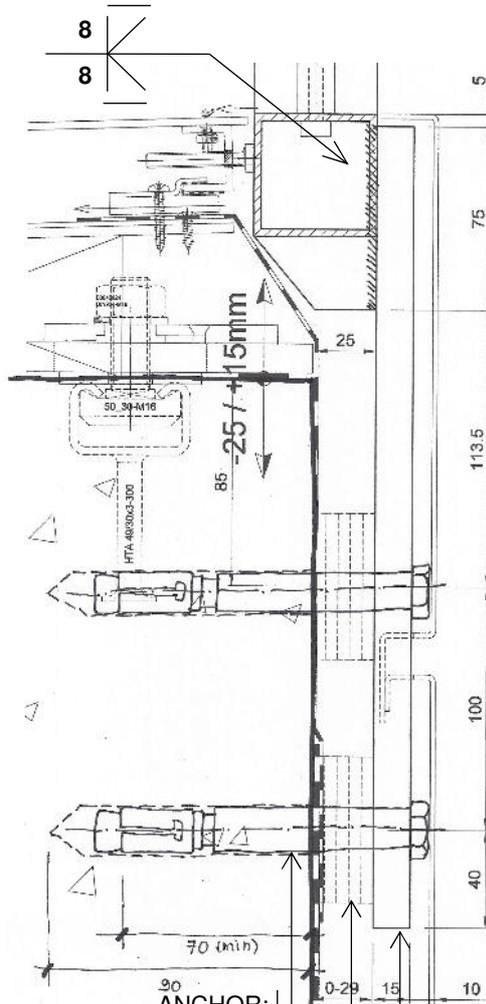


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## 4.2 Brackets and anchors

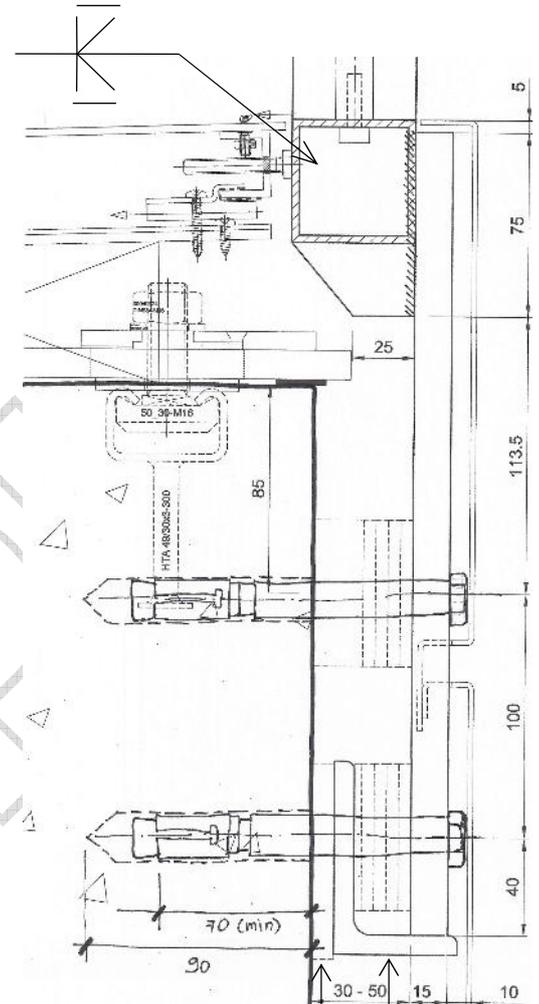
### 4.2.1 Details

#### Upstand in (-25mm ~ +4mm)



ANCHOR:  
 @ ENDS: 2 x HSL-3 M10/(65mm max)  
 @ INTERMEDIATE: 4 x HSL-3 M10/(65mm max)  
 METAL SHIM: 80mmx80mm  
 PLATE:  
 @ ENDS: 125x330x15mm / 6082 T6  
 @ INTERMEDIATE: 200x330x15mm / 6082 T6

#### Upstand out (+5mm ~ +25mm)



METAL SHIM: 80mmx100mm  
 SHELF ANGLE:  
 @ ENDS: 1 x 80mm - L100x50x8mm / 6060 T6  
 @ INT.: 2 x 80mm - L100x50x8mm / 6060 T6

### 4.2.2 Anchor Design Forces

From the results of structural analysis in section 4.1.3.

- i Upstand in (-25mm ~ +4mm) considering 29mm stand-off distance

$$V_{Sd} = 1.5 + 0.62 = 2.12 \text{ kN}$$

$$N_{Sd} = 2.48 \text{ kN}$$

- ii Upstand out (+5mm ~ +25mm) considering 15mm stand-off distance

$$V_{Sd} = 1.5 + 0.62 = 2.12 \text{ kN}$$

$$N_{Sd} = 2.48 + 2.12 \cdot 50 / (\%48) = 5.79 \text{ kN}$$



## Profis Anchor 2.5.2

www.hilti.ch

Company: Lindner Fassaden GmbH  
 Specifier:  
 Address:  
 Phone | Fax:  
 E-Mail:

Page:  
 Project:  
 Fastening Point:  
 Date:

1  
 The Comiche, London  
 BAL-102  
 18/05/2015

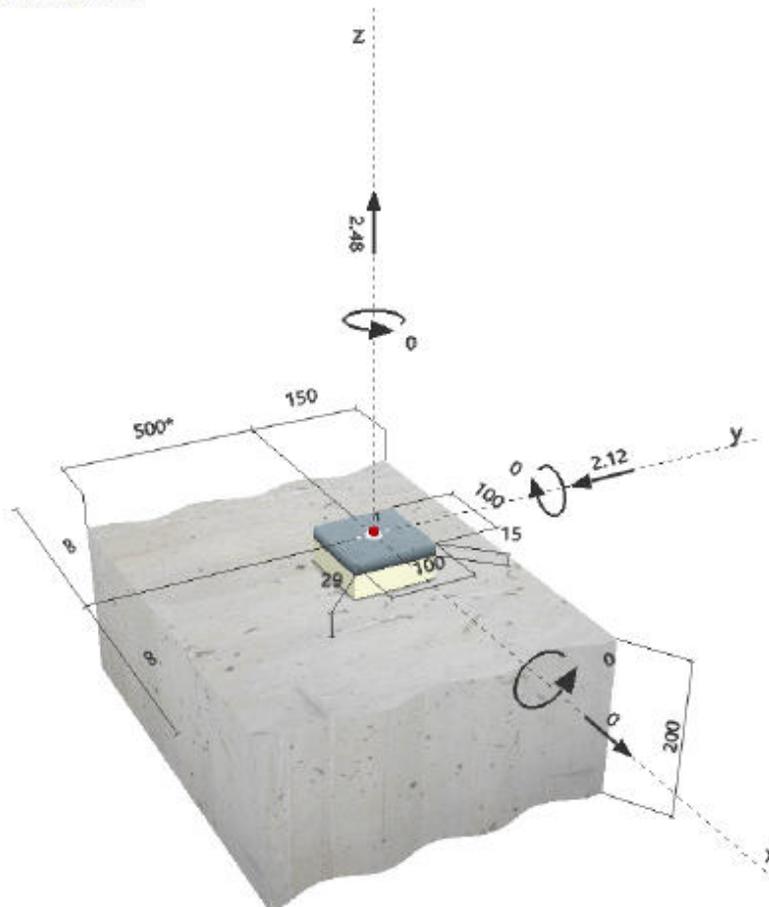
Specifier's comments:

## 1 Input data

Anchor type and size:	HSL-3 M10
Dynamic set or any suitable annular gap filling solution	
Effective embedment depth:	$h_{ef} = 70 \text{ mm}$ , $h_{nom} = 90 \text{ mm}$
Material:	8.8
Approval No.:	ETA 02/0042
Issued   Valid:	10/01/2013   10/01/2018
Proof:	Design method ETAG (No. 001 Annex C/2010)
Stand-off installation:	without clamping (anchor); restraint level (baseplate): 2.00; $e_b = 29 \text{ mm}$ ; $t = 15 \text{ mm}$ Hilti Grout: , multipurpose, $f_{c,Grout} = 30.00 \text{ N/mm}^2$
Baseplate:	$l_x \times l_y \times t = 100 \text{ mm} \times 100 \text{ mm} \times 15 \text{ mm}$ ; (Recommended plate thickness: not calculated)
Profile:	no profile
Base material:	cracked concrete, C35/45, $f_{cc} = 45.00 \text{ N/mm}^2$ ; $h = 200 \text{ mm}$
Reinforcement:	No reinforcement or Reinforcement spacing $\geq 150 \text{ mm}$ (any $\emptyset$ ) or $\geq 100 \text{ mm}$ ( $\emptyset \leq 10 \text{ mm}$ ) no longitudinal edge reinforcement



### Geometry [mm] & Loading [kN, kNm]



Input data and results must be checked for agreement with the existing conditions and for plausibility!  
 PROFIS Anchor (c) 2003-2009 Hilti AG, FL-9494 Schaan. Hilti is a registered trademark of Hilti AG, Schaan.